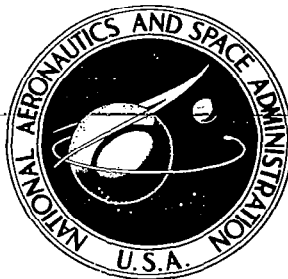


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**DESAP 2 - A STRUCTURAL DESIGN PROGRAM  
WITH STRESS AND BUCKLING CONSTRAINTS**

**Volume III: Program Listing**

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16. ABSTRACT <p>DFSAP 2 is a finite element program for computer-automated, minimum weight design of elastic structures with constraints on stresses (including local instability criteria) and buckling loads. No limits are placed on the number of load conditions for stress-constrained design, but only one of these load conditions can be chosen as the potential buckling load. A substantial portion of DESAP 2, particularly the analysis of the prebuckling state, is derived from the SOLID SAP finite element program developed at the University of California, Berkeley. The stress-constrained design is based on the classical stress ratio method, which drives the design toward a fully stressed state. The constraints on the buckling load are handled by solving the appropriate optimality criterion by successive iterations. During each iteration, the element sizes determined by the stress ratio method are used as the minimum size constraints. The element subroutines have been organized in a manner that permits the user to make additions and changes with a minimal programming effort. Consequently, DESAP 2 can readily be changed into a special-purpose program to handle the user's specific design requirements and failure criteria.</p> <p>DFSAP 2 is a companion program of DESAP 1: "A Structural Design Program with Stress and Displacement Constraints." With the exception of a few cards the same input data deck can be used for both programs.</p> <p>This is Volume III of three volumes.</p>					
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# TABLE OF CONTENTS

## VOLUME 3

<u>Name of Subroutine</u>	<u>Page</u>
Main Program	Q.1
INPUTJ	Q.7
INTERP	Q.8
ERROR	Q.8
ELTYPE	Q.9
NOELEM	Q.10
CALBAN	Q.10
ELGSUW	Q.11
VECTOR	Q.11
CROSS	Q.11
DOT	Q.11
REARAN	Q.12
UNITWT	Q.12
ELMULT	Q.12
INL	Q.13
INPUTZ	Q.13
DEVAR	Q.14
ELSTIF	Q.15
ELSTEW	Q.15
ADDSTF	Q.16
USOL	Q.17
PRINTD	Q.20
STRESS	Q.21
DPRINT	Q.22
MOVED	Q.22
STRSC	Q.23
ELGSTW	Q.24
BANAL	Q.25
ADGSTF	Q.27
MULBAN	Q.28
EIGJAC	Q.29
SFTF	Q.30
SMODE	Q.30
DERV	Q.31
DESIGN	Q.33
BDESIN	Q.36
DISP	Q.39
MESG	Q.40
TRUSS	Q.41
RUSS	Q.42
FTRUSS	Q.44
TGEOM	Q.45
DTRUSS	Q.46
JOHNS	Q.46

BEAM	Q.47
TEAM	Q.48
CBRT	Q.51
NEWBM	Q.52
SLAVE	Q.55
BGEOM	Q.56
DBEAM	Q.58
PLANE	Q.60
ELAW	Q.62
QUAD	Q.63
PLANCT	Q.68
POSINV	Q.68
FORMB	Q.69
PLNAX1	Q.70
DPLAN1	Q.73
PLNAX2	Q.75
DPLAN2	Q.77
SHEAR	Q.78
PANEL	Q.79
SPGEOM	Q.81
FPANEL	Q.82
DPANEL	Q.85
SHELL	Q.86
PLATE1	Q.87
QDCOS	Q.91
TDCOS	Q.91
QTSHEL	Q.93
SHLCD1	Q.96
SLST	Q.97
SLCCT	Q.98
SHLCT1	Q.101
SHLCT2	Q.101
SH2221	Q.102
DSHEL1	Q.103
SHELG1	Q.104
SHELG2	Q.105
SHLCD2	Q.106
BOUND	Q.107
CLAMP	Q.107
Dummy Element Subroutines	Q.110
(An element subroutine package may be replaced by the corresponding dummy subroutine if the element is not used in the program run.)	

```

C*****C,MA1N0000
C*****C,MA1N0010
C**
C** DESAP2---AN AUTOMATED DESIGN PROGRAM WITH STRESS AND
C** RIGIDITY CONSTRAINTS BASED ON SAP2 ANALYSIS PROGRAM
C**
C** BY J.K.HSIAO AND K.R. REDDY (MAY, 1976)
C**
C*****C,MA1N0020
C*****C,MA1N0030
C*****C,MA1N0040
C*****C,MA1N0050
C*****C,MA1N0060
C*****C,MA1N0070
C*****C,MA1N0080
C*****C,MA1N0090
COMMON /JUNK / HED(20),JUN(348)
COMMON/FM/DOO(5548)
COMMON/IIMJTS/ IR,IW,IP,I1,I2,I3,I4,I5,I6,I7,I8,I9,I10,I11,I12,I13
COMMON /ELPAR/ NPAP(14),NUMNP,MRAND,NFLIYP,N1,N2,N3,N4,N5,M1D1,NEQMA
1,NUMEL,NUMDV,M1,M2,M3,LL,LR,NFOR,NBLOCK
COMMON/CONTR/ ITCYCL,MCYCL,ISCALE,NSCALE,KSCALE,KOMVG,INFSM,IWTMIN,MA1N0150
IWTMIN,EPSI1,DELTA1,DELTA2,KPUNCH,LRHICK,NVFC,NMDDF,LR,ALPA,INDET
2,KPRINT,OMEGA,COEFF,SMAX,SMAX,NRICK,SE,IS,KOD,NRCIND
C*****C,MA1N0180
C-----PROGRAM CAPACITY CONTROLLED BY THE FOLLOWING THREE STATEMENTS
C*****C,MA1N0190
DIMENSION A(1500)
REAL*8 AD(750)
FOURVAL,FMCF (A(1),AD(1))
MTOT=1500
C*****C,MA1N0250
C-----INPUT-OUTPUT UNIT ASSIGNMENTS
C*****C,MA1N0260
C*****C,MA1N0270
IR=5
IW=6
IP=7
I1=1
I2=2
I3=3
IR=8
IO=9
I10=10
I11=11
I12=12
I13=13
C*****C,MA1N0400
C-----PROGRAM CONTROL DATA
C
C-----NUMNP = NUMBER OF NODE POINTS
C-----NFLIYP = NUMBER OF ELEMENT TYPES
C-----LI = NUMBER OF LOAD CONDITIONS
C-----NUMDV = NUMBER OF INDEPENDENT DESIGN VARIABLES
C-----MRAND = RANDOMIZE OF STIFFNESS MATRIX
C-----NUMEL = TOTAL NO. OF ELEMENTS
C-----MPAR = CONTROL PARAMETERS FOR EACH ELEMENT GROUP
C-----NFOR = NO. OF EQUATIONS PER BLOCK OF STIFFNESS MATRIX
C-----NBLOCK = NO. OF BLOCKS OF EQUILIBRIUM EQUATIONS
C-----MTOT = NO. OF STORAGE LOCATIONS OF COMMON ARRAY A
C*****C,MA1N0530
5 READ(IR,1000)HED,NUMNP,NFLIYP,LI,NUMDV
IF (NUMNP.EQ.0) STOP
WRITE (IW,2000)HED,NUMNP,NFLIYP,LI,NUMDV
C*****C,MA1N0570
C-----DESIGN CONTROL DATA
C

```

```

C-----KPRINT = PRINT OUT CODE                                MAIN0600
C              =0 NOBIAL DISPLACEMENTS NOT PRINTED           MAIN0610
C              =1 NOBIAL DISPLACEMENTS ARE PRINTED           MAIN0620
C-----KPUNCH = PUNCH OUT CODE FOR RESTART DECK              MAIN0630
C              =0 NO RESTART DECK PUNCHED                     MAIN0640
C              =1 PUNCHES RESTART DECK FOR DESIGN VARIABLE DATA MAIN0650
C-----LBUCK = CODE FOR BUCKLING CONSTRAINTS                  MAIN0660
C              =0 NO BUCKLING CONSTRAINT IS PRESENT           MAIN0670
C              =1 BUCKLING CONSTRAINT IS PRESENT FOR N'TH LOAD CONDITION MAIN0680
C-----NM00F = NO. OF LOWEST BUCKLING MODES FOR WHICH BUCKLING MAIN0690
C              CONSTRAINT APPLIES                               MAIN0700
C-----NVEC = NO. OF ITERATION VECTORS FOR BUCKLING CONSTRAINTS MAIN0710
C              (NM00F,LE,NVEC,LE,4 )                           MAIN0720
C-----IDFSM = CURRENT DESIGN NO.                              MAIN0730
C-----ICYCL = CURRENT CRITICAL DESIGN NO.                    MAIN0740
C-----MCYCL = MAX. ALLOWABLE NUMBER OF CRITICAL DESIGNS      MAIN0750
C-----KCONVG = DESIGN CONVERGENCE CODE                        MAIN0760
C              =1 DESIGN IS NOT CRITICAL                       MAIN0770
C              =2 DESIGN IS CRITICAL FOR BUCKLING CONSTRAINTS MAIN0780
C              =3 DESIGN IS CRITICAL FROM STRESS CONSTRAINTS  MAIN0790
C              =4 DESIGN IS ACCEPTABLE                         MAIN0800
C-----DELTA = DEFINES BAND OF CRITICAL DESIGNS AND           MAIN0810
C              USED IN CHECKING OPTIMALITY FOR BUCKLING CONSTRAINTS MAIN0820
C-----EPSIL = DEFINES ALLOWABLE WEIGHT INCREASE OVER WTMIN   MAIN0830
C-----WTMIN = MIN WEIGHT CRITICAL DESIGN                     MAIN0840
C-----IWTMIN = DESIGN NUMBER OF MIN. WT. CRITICAL DESIGN     MAIN0850
C-----ISCALE = SCALING OPERATION NUMBER                       MAIN0860
C-----NSCALE = MAX. ALLOWABLE NUMBER OF SUCCESSIVE SCALING OPERATIONS MAIN0870
C-----KSCALE = CODE FOR SCALING OPERATION                     MAIN0880
C              =-1 SCALING SHOULD NOT BE USED                 MAIN0890
C              =0 SCALING IS APPROXIMATE. REANALYSE SCALED STRUCTURE MAIN0900
C              =1 SCALING IS EXACT. STIFFNESS IS PROPORTIONAL TO SIZE MAIN0910
C              =2 SCALING IS EXACT. STIFFNESS IS PROPORTIONAL TO (SIZE)**2 MAIN0920
C              =3 SCALING IS EXACT. STIFFNESS IS PROPORTIONAL TO (SIZE)**3 MAIN0930
C              =4 SCALING IS EXACT. STIFFNESS IS PROPORTIONAL TO (SIZE)**4 MAIN0940
C              AND SO ON                                       MAIN0950
C-----ALPA = RELAXATION PARAMETER IN BUCKLING REDESIGN       MAIN0960
C-----COEFF = BUCKLING LOAD COEFFICIENT                      MAIN0970
C-----MODEIN = CODE FOR READING IN STARTING VECTORS FOR BUC. ANALYSIS MAIN0980
C              =0 GENERATE RANDOM NO.'S                       MAIN0990
C              =1 READ IN APPROXIMATE MODE SHAPES              MAIN1000
C-----INDFT = CODE FOR DETERMINACY OF THE STRUCTURE          MAIN1010
C              =0 INDETERMINATE STRUCTURE - BUCKLING ANALYSIS PERFORMED MAIN1020
C              FOR EVERY DESIGN                                MAIN1030
C              =1 STRUCTURE IS DETERMINATE - NO REANALYSIS FOR BUCKLING MAIN1040
C-----LRI = NO. OF DISPLACEMENT VECTORS THAT CAN BE STORED IN MAIN1050
C              COMMON AREA 'A' DURING BUCKLING DERIVATIVES COMPUTATION MAIN1060
C-----OMEGA = FACTOR FOR DECIDING POTENTIALLY ACTIVE BUCK. CONSTRAINTS MAIN1070
C-----SMAX = MAX. STRESS RATIO                                MAIN1080
C-----RMAX = RATIO COEFF/LOWEST BUCKLING LOAD                MAIN1090
C-----NBUCK = NO. OF POSSIBLE ACTIVE BUCKLING CONSTRAINTS   MAIN1100
C-----SF = SCALE FACTOR                                       MAIN1110
C*****MAIN1120
C              IDFSM=0                                          MAIN1130
C              ICYCL=0                                          MAIN1140
C              ISCALE=0                                         MAIN1150
C              WTMIN=1.0E20                                       MAIN1160
C              IWTMIN=0                                           MAIN1170
C              READ(1R,1001) MCYCL,NSCALE,KSCALE,DELTA,EPSIL,KPUNCH,KPRINT,LBUCK MAIN1180
C              TE(MSCALE,FO,0) NSCALE=3                          MAIN1190

```

```

      IF (DELTA.FO.O.O) DELTA=O.O5
      IF (FPSI.FO.O.O) FPSI=O.O1
      DELTA1=1.O-DELTA
      DELTA2=1.O+DELTA
      WRITE (IW,2001) NCYCL,KSCALE,DELTA,FPSI,LRUCK
      C*****
C-----INITIALISE UNIT WEIGHT (COEFF) C1FMTS
      C*****
      DO 100 I=1,NIMDV
      100 A(I)=O.O
      C*****
C-----MODE DATA----JO ARRAY STORED ON IR
      C*****
      M1=1
      M1=M1+NIMDV
      M2=M1+6*NIMNP
      M3=M2+NIMNP
      M4=M3+NIMNP
      M5=M4+NIMNP
      M6=M5+NIMNP
      IF (M6.GT.MTOT) CALL ERROR (M6-MTOT)
      CALL INPUTJ(A(N1),A(N2),A(N3),A(N4),A(N5),NIMNP,NFO,IR,IR,IW)
      C*****
C-----ELEMENT DATA --- UNIT STIFFNESS AND LOAD DATA ON I12
      C-----UNIT STRESS RECOVERY DATA ON IR
      C-----UNIT GEOMETRIC STIFFNESS DATA ON UNIT I11
      C*****
      MRAND=O
      NIMEL=O
      REWIND I11
      REWIND I12
      MRAND=O
      DO 900 M=1,NELIYP
      READ (IR,1002) NPAR
      WRITE (IR)NPAR
      NIMEL=NIMEL+NPAR(2)
      MTYPE=NPAR(1)
      IF (MTYPE.FO.7) MRAND=NPAR(2)
      900 CALL ELIYPF (A,M10T,MTYPE,I4)
      C*****
C-----WRITE UNIT WEIGHT ARRAY ON IR AND REARRANGE STORAGE OF ID
      C*****
      CALL UNITWT (A(M1),IR,NIMDV)
      J=6*NIMNP
      DO 121 I=1,J
      121 A(I)=A(NIMDV+I)
      C*****
C-----STORE HERE LOAD MULTIPLIERS---STORED ON UNIT I1
      C*****
      M1=1
      M2=M1+6*NIMNP
      CALL FLMULT(A(N2),I1,IR,IW,I1)
      M=1,L
      IF (LRUCK.FO.O) GO TO 110
      C*****
C-----READ RUCKLING CONTROL DATA
      C*****
      READ (IR,1003) COEFF1,MODEIN,NMODE,I,NVFC,ALPA,OMEGA
      IF (COEFF1.FO.O.O) COEFF1=1.O
      IF (NMODE.IF.O) NMODE=1

```

```

MAIN1200
MAIN1210
MAIN1220
MAIN1230
MAIN1240
MAIN1250
MAIN1260
MAIN1270
MAIN1280
MAIN1290
MAIN1300
MAIN1310
MAIN1320
MAIN1330
MAIN1340
MAIN1350
MAIN1360
MAIN1370
MAIN1380
MAIN1390
MAIN1400
MAIN1410
MAIN1420
MAIN1430
MAIN1440
MAIN1450
MAIN1460
MAIN1470
MAIN1480
MAIN1490
MAIN1500
MAIN1510
MAIN1520
MAIN1530
MAIN1540
MAIN1550
MAIN1560
MAIN1570
MAIN1580
MAIN1590
MAIN1600
MAIN1610
MAIN1620
MAIN1630
MAIN1640
MAIN1650
MAIN1660
MAIN1670
MAIN1680
MAIN1690
MAIN1700
MAIN1710
MAIN1720
MAIN1730
MAIN1740
MAIN1750
MAIN1760
MAIN1770
MAIN1780
MAIN1790

```



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      IF (NMORDE.GT.2) NMORDE=2                                MAIN1800
      IF (NVFC.L1.NMORDE) NVFC=2                                MAIN1810
      IF (NVFC.GT.2) NVFC=2                                      MAIN1820
      WRITE (IW,2005) COEFF1,MORDE1,N,MORDE,INDET,NVFC,ALPA,OMEGA MAIN1830
      IF (M,I,I,NVFC) M=NVFC                                    MAIN1840
110  NFORA=(MTOT-4*I1)/(MRAND+L1)*4)                            MAIN1850
      NFOR=M101/(MRAND+M)*4+1)                                  MAIN1860
      IF (NFORA.L1,NFOR) NFOR=NFORA                             MAIN1870
      IF (NFOR.GT.NFO) NFOR=NFO                                  MAIN1880
      NRLOCK=(NFO-1)/NFOR+1                                     MAIN1890
C*****MAIN1900
C-----MODAL LOADS --- STOPPED ON UNIT 112                     MAIN1910
C*****MAIN1920
      N2=N2+6*I1                                                MAIN1930
      ND3=(N3-1)/2+1                                           MAIN1940
      N4=(ND2+NFOR*I1)*2                                       MAIN1950
      IF (N4.GT.MTOT) CALL FPROR(N4-MTOT)                      MAIN1960
      CALL INI(A(N1),A(N2),AD(ND2),NMOR,NFOR,L1,IP,IW,I12)    MAIN1970
C*****MAIN1980
C-----READ OR GENERATE BUCKLING MODE SHAPES - WRITE ON UNIT 113 MAIN1990
C*****MAIN2000
      IF (LBUCK.NF.C) CALL INP112(AD(N1),NFOR,NRLOCK,NVFC,MORDE1,N,NFO MAIN2010
      I 113,IR)                                                MAIN2020
C*****MAIN2030
C-----DESIGN VARIABLE DATA---ADD AND AMIN ON I1             MAIN2040
C*****MAIN2050
      N2=N1+NUMOV                                              MAIN2060
      CALL DEVAR (A(N1),A(N2),NUMOV,I1,IR,IW)                  MAIN2070
      WRITE (IW,2002)                                           MAIN2080
      WRITE (IW,2003)NFO,MRAND,NFOR,NRLOCK                     MAIN2090
      WRITE (IW,2002)                                           MAIN2100
C*****MAIN2110
C-----FORM ELEMENT STIFFNESS AND LOAD VECTORS AND WRITE ON UNIT 110 MAIN2120
C*****MAIN2130
      995 N1=1                                                  MAIN2140
      CALL ELSTIF (A(N1),NUMOV,NUMEL,I1,I2,I12)                MAIN2150
C*****MAIN2160
C-----FORM STRUCTURAL STIFFNESS AND LOAD VECTORS AND WRITE ON UNIT 110 MAIN2170
C*****MAIN2180
      NF2R=2*NFOR                                              MAIN2190
      ND2=N1+NF2R*MRAND)                                       MAIN2200
      ND3=ND2+NF2R*I1                                           MAIN2210
      N3=(ND3-1)*2+1                                           MAIN2220
      N4=N3+4*I1                                                MAIN2230
      IF (N4.GT.MTOT) CALL FPROR(N4-MTOT)                      MAIN2240
      CALL ADDSTF(AD(N1),AD(ND2),A(N3),NUMEL,NRLOCK,NF2R,L1,MRAND, MAIN2250
      I 11,I2,I9,I10,I12)                                     MAIN2260
C*****MAIN2270
C-----SOLVE FOR DISPLACEMENT UNKNOWNNS                      MAIN2280
C*****MAIN2290
      NSR=(MRAND+L1)*NFOR                                       MAIN2300
      N2=N1+NFOR                                                MAIN2310
      ND2=N2/2+1                                                MAIN2320
      ND3=ND2+NSR                                              MAIN2330
      CALL USOL (A(N1),AD(ND2),AD(ND3),NFOR,MRAND,L1,NRLOCK,NSR,I10,I3, MAIN2340
      I 10,I2,IW)                                              MAIN2350
C*****MAIN2360
C-----PRINT MODAL DISPLACEMENTS                             MAIN2370
C*****MAIN2380
      N2=N1+NUMINP*6                                           MAIN2390

```

```

      N3=N2+6*IL
      MD3=MD2/2+1
      WRITE(1W,2004)IDFSN
      CALL PRINTN(A(N1),A(N2),AD(MD3),NFOR,NUMMP,LL,
1  NRLOCK,NFO,I2,I3,IW,I,KPR)MT
C*****
C-----COMPUTE STRESSES AND CAPRY OUT FULLY STRESSED DESIGN
C-----COMPUTE GEOMETRIC STIFFNESS MATRIX AND WRITE ON UNIT 13
C*****
      M1=1
      M2=M1+NUMMDV
      M3=M2+NUMMDV
      M1=M3+NUMMDV
      N2=M1+4*IL
      MD2=MD2/2+1
      N2=(MD2+NFOR*IL)*2+1
      LR=(MTOT-N3)/NFO
      IF(LR,GE,1) GO TO 31
      MM=NFO+M3
      CALL FPROR(MM-MTOT)
31 IF(LR,GT,1)LR=1
      CALL STRESS(A(M1),A(M2),A(M3),A(N1),AD(MD2),A(N3),LL,LR,NFO,NUMMDV,
1  NFOR,A,MTOT,LRUCK,INDET,IDFSN,I1,I2,I3,I4,I1,IW)
C*****
C-----BUCKLING ANALYSIS
C*****
      MTOT2=MTOT/2
      IF(LRUCK,NFO) CALL BANA1(A,AD,MTOT,MTOT2)
      IF(NCYC1,F0,0) GO TO 996
C*****
C-----EVALUATE CURRENT DESIGN AND PERFORM REDSIGN
C*****
      M1=1
      M2=M1+NUMMDV
      M3=M2+NUMMDV
      M4=M3+NUMMDV
      M5=M4+NUMMDV
      I1=18
      IF(KPRINT,F0,0) GO TO 835
      I1=10
      REWIND 11
835 CALL DESIGN(A(N1),A(N2),A(N3),A(N4),A(N5),NUMMDV,LL,I1)
      IDFSN=IDFSN+1
      IF(KDNV6,F0,4) GO TO 996
      IF(NRUCK,F0,0) GO TO 995
C*****
C-----BUCKLING DERIVATIVES
C*****
      N=4*IL
      REWIND 11
      READ(11) (A(I),I=1,N)
      LR1=(MTOT-NUMMDV-NFOR*NUMPDE*2)/(NFO+NUMMDV)
      M=2*NUMMDV+NFO+MTOT*NR*NUMPDE*2-MTOT
      IF(LR1,I1,1) CALL FPROR(M)
      IF(LR1,GT,NRUCK)LR1=NRUCK
      M2=M1+NUMMDV
      M3=M2+NUMMDV*IL
      M4=M3+NFOR*IL
      MD4=M4/2+1
      CALL DFRV(A(N1),A(N2),A(N3),AD(MD4),LR1,NRUCK,NFOR,NRLOCK,NFO,

```

```

      1 NIMDV,NMODE,LL,NMFL,11,12,13,112,1R,14)
C *****
C ----PREFORM BUICKLING DEFESIGN
C *****
      M1=1
      M2=M1+NIMDV
      M3=M2+NIMDV*NBUICK
      M4=M3+NIMDV
      M5=M4+NIMDV
      M6=M5+NIMDV
      M7=M6+NIMDV
      M8=M7+NIMDV
      N=(6+NBUICK)*NIMDV+4*11-MTOP
      IF(N.GT.0) CALL ERRPOP(N)
      CALL DEFESIN (A(M1),A(M2),A(M3),A(M4),A(M5),A(M6),A(M7),A(M8),
      1 NIMDV,10,LL,NBUICK)
      IF(KONVGR.NF.4) GO TO 995
      996 STOP
1000 FORMAT(20A4/415)
1001 FORMAT(315,2F10.0,315)
1002 FORMAT(1415)
1003 FORMAT(F10.0,415,2F10.0)
2000 FORMAT(1H1,20A4//
      . 2RH NUMBER OF NODAL POINTS =,15/
      . 2RH NUMBER OF ELEMENT TYPES =,15/
      . 2RH NUMBER OF LOAD CASES =,15/
      . 2RH NUMBER OF DES. VARIABLES =,15 )
2001 FORMAT(// 22H DESIGN CONTROL DATA //
      1 9H NCYCL =,15/
      2 9H KSCALE=,15/
      3 9H DELTA =,F12.4/
      4 9H EPSIL =,F12.4/
      5 9H LBUICK =,15 )
2002 FORMAT(//)
2003 FORMAT(34H TOTAL NUMBER OF EQUATIONS =,15,
      1 /34H RANDWIDTH =,15,
      2 /34H NUMBER OF EQUATIONS IN A BLOCK =,15,
      3 /34H NUMBER OF BLOCKS =,15)
2004 FORMAT(31H) *****/
      1 26H ANALYSIS OF DESIGN NUMBER,14 /
      2 31H *****//)
2005 FORMAT(// 22H BUICKLING CONTROL DATA //
      . 9H COEFFT =,F10.5/
      1 9H MDEFIN =,15/
      2 9H NMODE =,15/
      3 9H JMODET =,15/
      4 9H NVFC =,15/
      5 9H ALPA =,F10.5 /
      6 9H OMEGA =,F10.5)
      END

```

```

MAIN3000
MAIN3010
MAIN3020
MAIN3030
MAIN3040
MAIN3050
MAIN3060
MAIN3070
MAIN3080
MAIN3090
MAIN3100
MAIN3110
MAIN3120
MAIN3130
MAIN3140
MAIN3150
MAIN3160
MAIN3170
MAIN3180
MAIN3190
MAIN3200
MAIN3210
MAIN3220
MAIN3230
MAIN3240
MAIN3250
MAIN3260
MAIN3270
MAIN3280
MAIN3290
MAIN3300
MAIN3310
MAIN3320
MAIN3330
MAIN3340
MAIN3350
MAIN3360
MAIN3370
MAIN3380
MAIN3390
MAIN3400
MAIN3410
MAIN3420
MAIN3430
MAIN3440
MAIN3450
MAIN3460
MAIN3470
MAIN3480
MAIN3490

```

```

      SRRRDIIME INPUT(J(1D,X,Y,Z,T,NUMNP,NFO,IR,IR,TW) MA1N3500
C-----READ OR GENERATE MODAL POINT DATA MA1N3510
C-----***** MA1N3520
      DIMENSION X(NUMNP),Y(NUMNP),Z(NUMNP),ID(NUMNP,6),T(NUMNP) MA1N3530
      RWJND IR MA1N3540
      WRITE(IW,2000) MA1N3550
      WRITE(IW,2001) MA1N3560
      NDI,N=0 MA1N3570
      10 READ (IR,1000)N,(ID(N,I),I=1,6),X(N),Y(N),Z(N),KN,T(N) MA1N3580
      WRITE(IW,2002)N,(ID(N,I),I=1,6),X(N),Y(N),Z(N),KN,T(N) MA1N3590
C-----***** MA1N3600
C-----CHECK IF GENERATION IS REQUIRED MA1N3610
C-----***** MA1N3620
      IF(NDI,NFO,N) GO TO 50 MA1N3630
      DO 20 I=1,6 MA1N3640
      IF(ID(N,I),FO,N,AND,ID(NDI,N,I),T,N) ID(N,I)=ID(NDI,N,I) MA1N3650
      20 CONTINUE MA1N3660
      IF(KN,FO,N) GO TO 50 MA1N3670
      NIIM=(N-NDI,N)/KN MA1N3680
      NIIM=NIIM-1 MA1N3690
      IF(NIIM,1,1) GO TO 50 MA1N3700
      XNIIM=NIIM MA1N3710
      DX=(X(N)-X(NDI,N))/XNIIM MA1N3720
      DY=(Y(N)-Y(NDI,N))/XNIIM MA1N3730
      DZ=(Z(N)-Z(NDI,N))/XNIIM MA1N3740
      DT=(T(N)-T(NDI,N))/XNIIM MA1N3750
      K=NDI,N MA1N3760
      DO 30 J=1,NIIM MA1N3770
      KK=K MA1N3780
      K=K+KN MA1N3790
      X(K)=X(KK)+DX MA1N3800
      Y(K)=Y(KK)+DY MA1N3810
      Z(K)=Z(KK)+DZ MA1N3820
      T(K)=T(KK)+DT MA1N3830
      ID(K,I)=ID(KK,I) MA1N3840
      IF(ID(K,I),GT,1) ID(K,I)=ID(KK,I)+KN MA1N3850
      30 CONTINUE MA1N3860
      50 NDI,N=N MA1N3870
      IF(N,NF,NUMNP) GO TO 10 MA1N3880
C-----***** MA1N3890
C-----PRINT ALL MODAL POINT DATA MA1N3900
C-----***** MA1N3910
      WRITE(IW,2003) MA1N3920
      WRITE(IW,2004) MA1N3930
      WRITE(IW,2005) (N,(ID(N,I),I=1,6),X(N),Y(N),Z(N),T(N),N=1,NUMNP) MA1N3940
C-----***** MA1N3950
C-----NUMBER UNKNOWNMS AND SET MASTER NODES NEGATIVE MA1N3960
C-----***** MA1N3970
      NFO=0 MA1N3980
      DO 60 N=1,NUMNP MA1N4000
      DO 60 I=1,6 MA1N4010
      ID(N,I)=I*ABS(ID(N,I)) MA1N4020
      IF(ID(N,I)-1) 57,58,59 MA1N4030
      57 NFO=NFO+1 MA1N4040
      ID(N,I)=NFO MA1N4050
      GO TO 60 MA1N4060
      58 ID(N,I)=0 MA1N4070
      GO TO 60 MA1N4080
      60 MA1N4090

```

```

59 10(N,I)=-10(N,I) MAIN4100
60 CONTINUE MAIN4110
WRITE(14,2004) (N,(10(N,I),I=1,6),N=1,NIMMP) MAIN4120
WRITE(14) 10 MAIN4130
RETURN MAIN4140
1000 FORMAT (7I5,3F10.0,15,F10.0) MAIN4150
2000 FORMAT(// 23H MODAL POINT INPUT DATA ) MAIN4160
2001 FORMAT (5HMODUL,3X,24HBOUNDARY CONDITION CODES,3X, MAIN4170
130H/-----MODAL POINT COORDINATES-----//.
27H NUMBER,2X,1HX,4X,1HY,4X,1H7,3X,2HXX,3X,2HYY,3X,2HZZ,12X,
31HX,12X,1HY,12X,1H7,12X,1H1/) MAIN4180
2002 FORMAT (15,6I5,3F13.3,15,F13.3) MAIN4190
2003 FORMAT (// 21H GENERATED MODAL DATA) MAIN4200
2004 FORMAT (// 17H EQUATION NUMBERS// MAIN4210
1 35H M Y 7 XX YY ZZ / (7I5)) MAIN4220
2005 FORMAT (15,6I5,4F13.3) MAIN4230
END MAIN4240
MAIN4250
MAIN4260

SUBROUTINE INTERP (F,FF,NIMTC,NIMMAT,NIM1,NIM2,NT,MAT,TEMP) MAIN4270
C***** MAIN4280
C-----INTERPOLATES MATERIAL PROPERTIES FOR AVERAGE ELEMENT TEMPERATURE MAIN4290
C***** MAIN4300
IMPLICIT REAL*8 (A-H,O-Z)
REAL*4 F MAIN4310
DIMENSION F(NIMTC,NIM1,NIMMAT),FF(NIM2) MAIN4320
IF(NT.NE.1) GO TO 220 MAIN4330
DO 210 KK=1,NIM2 MAIN4340
210 FF(KK)=F(1,KK+1,MAT) MAIN4350
GO TO 240 MAIN4360
220 DO 230 I=2,NT MAIN4370
I1=1 MAIN4380
T1=F(I1,I,MAT) MAIN4390
T2=F(I,I,MAT) MAIN4400
IF(T2.GE.TEMP) GO TO 240 MAIN4410
230 CONTINUE MAIN4420
240 RI=(T2-TEMP)/(T2-T1) MAIN4430
RJ=(TEMP-T1)/(T2-T1) MAIN4440
DO 250 KK=1,NIM2 MAIN4450
250 FF(KK)=F(I1,KK+1,MAT)*RI+F(I,KK+1,MAT)*RJ MAIN4460
260 RETURN MAIN4470
END MAIN4480
MAIN4490

SUBROUTINE FPROP(N) MAIN4500
COMMON/INTTS/ IR,IW,JP,I1,I2,I3,IR,I9,I10,I11,I12,I13
WRITE (14,2000) N MAIN4510
STOP MAIN4520
2000 FORMAT (// 20H STORAGE EXCEEDED BY ,16) MAIN4530
END MAIN4540
MAIN4550

```

```

SUBROUTINE FLTYPE (A,MTOT,MTYPE,IW)
C*****MAIN4560
C-----CALL APPROPRIATE ELEMENT SUBROUTINE TO DEVELOP ELEMENT MATRICES
C*****MAIN4570
C*****MAIN4580
C*****MAIN4590
      DIMENSION A(MTOT)
      GO TO (1,2,3,4,5,6,7,8),MTYPE
C*****MAIN4600
C*****MAIN4610
C-----THREE DIMENSIONAL TRUSS ELEMENTS
C*****MAIN4620
C*****MAIN4630
C*****MAIN4640
      1 CALL TRUSS (A,MTOT)
      GO TO 900
C*****MAIN4650
C*****MAIN4660
C*****MAIN4670
C-----THREE DIMENSIONAL BEAM ELEMENTS
C*****MAIN4680
C*****MAIN4690
      2 CALL BEAM (A,MTOT)
      GO TO 900
C*****MAIN4700
C*****MAIN4710
C*****MAIN4720
C-----PLANE STRESS ELEMENTS
C*****MAIN4730
C*****MAIN4740
      3 CALL PLANE (A,MTOT)
      GO TO 900
C*****MAIN4750
C*****MAIN4760
C*****MAIN4770
C-----SHEAR PANEL ELEMENTS
C*****MAIN4780
C*****MAIN4790
      4 CALL SHEAR (A,MTOT)
      GO TO 900
C*****MAIN4800
C*****MAIN4810
C*****MAIN4820
C-----PROVISION FOR OTHER TYPES OF ELEMENTS
C*****MAIN4830
C*****MAIN4840
      5 CALL NOELF(MTYPE,0,IW)
      GO TO 900
C*****MAIN4850
C*****MAIN4860
C*****MAIN4870
C-----PLATE/SHELL ELEMENTS
C*****MAIN4880
C*****MAIN4890
      6 CALL SHELL (A,MTOT)
      GO TO 900
C*****MAIN4900
C*****MAIN4910
C*****MAIN4920
C-----BOUNDARY ELEMENTS
C*****MAIN4930
C*****MAIN4940
      7 CALL BOUND (A,MTOT)
      GO TO 900
C*****MAIN4950
C*****MAIN4960
C*****MAIN4970
C-----PROVISION FOR OTHER TYPES OF ELEMENTS
C*****MAIN4980
C*****MAIN4990
      8 CALL NOELF(MTYPE,0,IW)
      900 RETURN
      END
C*****MAIN5000
C*****MAIN5010
C*****MAIN5020

```

```

      SUBROUTINE NPELEM (MTYPE,KODE,IW)
      C*****
      C-----PRINT THE MESSAGE THAT REQUIRED ELEMENT SUBROUTINE IS MISSING
      C*****
      WRITE (IW,100) MTYPE
      IF (KODE.NE.0) WRITE (IW,101) KODE
      STOP
      100 FORMAT (//46H THE FOLLOWING ELEMENT HAS NOT BEEN PROGRAMED:
      1      14H ELEMENT TYPE=,I2)
      101 FORMAT( 14H CONSTR CODE=,I2)
      END

```

```

      SUBROUTINE CALBAN(NDIF,LM,S,P,ST,IT,MU,NV,NS,ND,NW,IDVAR,IFX,FRC)
      C*****
      C-----CALCULATE BANDWIDTH OF STRUCTURE STIFFNESS MATRIX
      C-----WRITE UNIT STRESS RECOVERY MATRICES AND STRESS-CORRECTION MATRICES
      C      ON TAPE 18
      C-----WRITE UNIT STIFFNESS AND LOAD VECTOR ON TAPE 112
      C*****
      IMPLICIT REAL*8 (A-H,O-Z)
      REAL*4 FRC
      DIMENSION LM(ND),S(ND,MU),P(ND,4,NV),ST(NS,ND,MU),TT(NS,4,NW),
      1 IS(6)
      COMMON/ELPAR/MPAR(14),MUHMP,MHAMB,IFLP(17)
      COMMON/UNITSF/ IR,IW,IP,II,I2,I3,IR,I9,I10,I11,I12,I13
      MIN=100000
      MAX=0
      DO 800 L=1,ND
      IF (LM(L).EQ.0) GO TO 800
      IF (LM(L).GT.MAX) MAX=LM(L)
      IF (LM(L).LT.MIN) MIN=LM(L)
      800 CONTINUE
      NDIF=MAX-MIN+1
      IF (NDIF.GT.MHAMB) MHAMB=NDIF
      IRD=6+ND*(1+MU*ND+NV*4)
      IS(1)=MU
      IS(2)=NW
      IS(3)=NS
      IS(4)=ND
      IS(5)=IDVAR
      IS(6)=IFX
      WRITE(IR ) IS,FRC,LM,ST,IT
      IS(1)=IRD
      IS(2)=MU
      IS(3)=NV
      IS(4)=ND
      WRITE(I12) IS,FRC,LM,S,P
      RETURN
      END

```

```

      SUBROUTINE FLAGSHW(G,NSG,ND,MG,111)
      *****
      C-----WRITE ELEMENT UNIT GEOMETRIC STIFFNESS MATRICES ON TAPE 111
      *****
      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION G(ND,ND,MG),NSG(MG)
      WRITE(111) MG,G,NSG
      RETURN
      END

```

```

      MAIN5510
      *****
      MAIN5520
      *****
      MAIN5530
      *****
      MAIN5540
      *****
      MAIN5550
      *****
      MAIN5560
      *****
      MAIN5570
      *****
      MAIN5580
      *****
      MAIN5590

```

```

      SUBROUTINE VECTOR(V,XI,YI,ZI,XJ,YJ,ZJ)
      *****
      C-----CALCULATE COMPONENTS OF A VECTOR
      *****
      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION V(4)
      X=XI-YI
      Y=YI-YI
      Z=ZJ-YI
      V(4)=DSORT(X*X+Y*Y+Z*Z)
      V(3)=Z/V(4)
      V(2)=Y/V(4)
      V(1)=X/V(4)
      RETURN
      END

```

```

      MAIN5600
      *****
      MAIN5610
      *****
      MAIN5620
      *****
      MAIN5630
      *****
      MAIN5640
      *****
      MAIN5650
      *****
      MAIN5660
      *****
      MAIN5670
      *****
      MAIN5680
      *****
      MAIN5690
      *****
      MAIN5700
      *****
      MAIN5710
      *****
      MAIN5720
      *****
      MAIN5730
      *****
      MAIN5740

```

```

      SUBROUTINE CROSS(A,R,C)
      *****
      C-----CROSS PRODUCT OF TWO VECTORS
      *****
      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION A(4),R(4),C(4)
      X=A(2)*R(3)-A(3)*R(2)
      Y=A(3)*R(1)-A(1)*R(3)
      Z=A(1)*R(2)-A(2)*R(1)
      C(4)=DSORT(X*X+Y*Y+Z*Z)
      C(3)=Z/C(4)
      C(2)=Y/C(4)
      C(1)=X/C(4)
      RETURN
      END

```

```

      MAIN5750
      *****
      MAIN5760
      *****
      MAIN5770
      *****
      MAIN5780
      *****
      MAIN5790
      *****
      MAIN5800
      *****
      MAIN5810
      *****
      MAIN5820
      *****
      MAIN5830
      *****
      MAIN5840
      *****
      MAIN5850
      *****
      MAIN5860
      *****
      MAIN5870
      *****
      MAIN5880
      *****
      MAIN5890

```

```

      REAL FUNCTION DOTPR(A,R)
      *****
      C-----DOT PRODUCT OF TWO VECTORS
      *****
      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION A(4),R(4)
      DOT=A(1)*R(1)+A(2)*R(2)+A(3)*R(3)
      RETURN
      END

```

```

      MAIN5900
      *****
      MAIN5910
      *****
      MAIN5920
      *****
      MAIN5930
      *****
      MAIN5940
      *****
      MAIN5950
      *****
      MAIN5960
      *****
      MAIN5970
      *****
      MAIN5980

```



```

SUBROUTINE REARRAN(S,SS,NM1,NM2,NM3,N1,N2,N3,N4)
C*****
C-----REARRANGE MATRIX S
C*****
IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION S(NM1,NM2,NM3),SS(N4)
II=0
DO JO K=1,NM3
DO IO J=1,N2
DO II I=1,N1
11 SS(II+1)=S(I,J,K)
IO II=J+NM1
RETURN
END
MAIN5990
MAIN6000
MAIN6010
MAIN6020
MAIN6030
MAIN6040
MAIN6050
MAIN6060
MAIN6070
MAIN6080
MAIN6090
MAIN6100
MAIN6110
MAIN6120

```

```

SUBROUTINE UNITWT(IWT,IR,NIMDV)
C*****
C-----WRITE UNIT WEIGHT ON TAPE IR
C*****
DIMENSION IWT(NIMDV)
WRITE(IR)IWT
RETURN
END
MAIN6130
MAIN6140
MAIN6150
MAIN6160
MAIN6170
MAIN6180
MAIN6190
MAIN6200

```

```

SUBROUTINE FLMULT(STR,LL,IR,IW,II)
C*****
C-----READ IN STRUCTURE LOAD MULTIPLIERS
C*****
DIMENSION STR(4,II)
REWIND II
WRITE(IW,2000)
DO 50 I=1,II
READ(IR,1002) (STP(I,L),L=1,4)
50 WRITE(IW,2002) I,(STR(I,L),L=1,4)
WRITE(II) STR
RETURN
1002 FORMAT (4F10.0)
2000 FORMAT (//10H STRUCTURE LOAD MULTIPLIERS/
. 10H LOAD CASE,9X,1HA,9X,1HB,9X,1HC,9X,1HD/ )
2002 FORMAT (1A,7X,4F10.3)
END
MAIN6210
MAIN6220
MAIN6230
MAIN6240
MAIN6250
MAIN6260
MAIN6270
MAIN6280
MAIN6290
MAIN6300
MAIN6310
MAIN6320
MAIN6330
MAIN6340
MAIN6350
MAIN6360
MAIN6370

```

```

SUBROUTINE INI(IID,TR,R,NIMNP,NFOR,LL,IR,IW,112)
C*****
C-----INPUT NODAL LOADS
C*****
      IMPLICIT REAL*8 (A-H,O-Z)
      REAL*4 TR
      DIMENSION ID(NIMNP,6),TR(6,LL),R(NFOR,LL)
      COMMON/ILINK/R(6),ILINK(356)
      KSHF=0
      WRITE(IW,2002)
      DO 750 I=1,NFOR
      DO 750 K=1,LL
750  R(I,K)=0.0
      DO 900 NN=1,NIMNP
      DO 100 J=1,6
      DO 100 J=1,LL
100  TR(I,J)=0.0
      IF(NN,FO,1) GO TO 300
150  IF(N,NF,NN) GO TO 400
      DO 200 I=1,6
200  TR(I,I)=R(I)
300  READ(IR,1001) N,I,R
      IF(N,FO,0) GO TO 150
      WRITE(IW,2001) N,I,R
      GO TO 150
400  DO 800 J=1,6
      JJ=ID(NN,J)-KSHF
      IF(JJ) 800,800,500
500  DO 600 K=1,LL
600  R(I,K)=TR(J,K)
610  IF(JI,NF,NFOR) GO TO 800
      WRITE(112) R
      KSHF=KSHF+NFOR
      DO 700 I=1,NFOR
      DO 700 K=1,LL
700  R(I,K)=0.0
800  CONTINUE
900  CONTINUE
      WRITE(112) R
      RETURN
1001 FORMAT(2I5,7F10.4)
2001 FORMAT(2I5,4F13.3)
2002 FORMAT(//10H NODAL POINT LOADS // 10H NODE LOAD,23X
. 14HAPPLIED LOADS / 10H MIN. CASE ,9X, 2HRX, 11X,
. 2HRX,11X,2HR7,11X,2HMX,11X,2HMY,11X,2HMZ )
      END

```

```

MAIN6380
MAIN6390
MAIN6400
MAIN6410
MAIN6420
MAIN6430
MAIN6440
MAIN6450
MAIN6460
MAIN6470
MAIN6480
MAIN6490
MAIN6500
MAIN6510
MAIN6520
MAIN6530
MAIN6540
MAIN6550
MAIN6560
MAIN6570
MAIN6580
MAIN6590
MAIN6600
MAIN6610
MAIN6620
MAIN6630
MAIN6640
MAIN6650
MAIN6660
MAIN6670
MAIN6680
MAIN6690
MAIN6700
MAIN6710
MAIN6720
MAIN6730
MAIN6740
MAIN6750
MAIN6760
MAIN6770
MAIN6780
MAIN6790
MAIN6800
MAIN6810
MAIN6820
MAIN6830

```

```

SUBROUTINE INPUT7(NFOR,NR,NCK,NVEC,MODFIN,NFO,113,IR)
C*****
C-----READ IN OR GENERATE THE COORDINATE VECTORS AND WRITE ON TAPE 113
C-----C1=(2**31)-1
C-----C2=1/(2**31)
C*****
      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION 7(NFOR,NVEC)
      DATA C1/2147483647.00/,C2/7342000000000000.0/
      REWIND 113

```

```

MAIN6840
MAIN6850
MAIN6860
MAIN6870
MAIN6880
MAIN6890
MAIN6900
MAIN6910
MAIN6920
MAIN6930

```

```

      77=1
      L1=NEFOR
      DO 200 I=1,NMLDCK
      DO 100 J=1,NFOR
      DO 100 J=1,NVFC
100  Z(I,J)=0.0
      IF(I,FO,NMLDCK) L1=NFO-NFOR*(NMLDCK-1)
      IF(MODEIN,NF,0) GO TO 300
C*****
C-----GENERATE RANDOM STARTING VECTORS USING Z7 AS SEED
C-----INITFORM PSUEDO-RANDOM NUMBER GENERATOR BETWEEN-0.5 AND 0.5
C*****
      DO 500 I=1,I1
      DO 500 J=1,NVFC
      Z7=DMOD(16807,4096)*Z7,C1)
      500  Z(I,J)=Z7*C2-0.5
      GO TO 250
C*****
C-----READ IN STARTING COORDINATE VECTORS
C*****
      200 READ(IP,2000 ) ((Z(I,J),J=1,NVFC),I=1,I1)
      250 CONTINUE
      WRITE(I13) Z
      200 CONTINUE
      RETURN
2000  FORMAT(8F10.5)
      END
      MAIN6940
      MAIN6950
      MAIN6960
      MAIN6970
      MAIN6980
      MAIN6990
      MAIN7000
      MAIN7010
      MAIN7020
      MAIN7030
      MAIN7040
      MAIN7050
      MAIN7060
      MAIN7070
      MAIN7080
      MAIN7090
      MAIN7100
      MAIN7110
      MAIN7120
      MAIN7130
      MAIN7140
      MAIN7150
      MAIN7160
      MAIN7170
      MAIN7180
      MAIN7190
      MAIN7200

      SUBROUTINE DEVAR(ADLD,AMIN,NUMDV,I1,I2,IW)
C*****
C-----READ OR GENERATE DESIGN VARIABLE DATA
C*****
      DIMENSION ADLD(NUMDV),AMIN(NUMDV)
      NDL=0
      WRITE (IW,100)
      9 READ(IR,101)N,ADLD(N),AMIN(N)
      NN=N-1
      IF(NN,FO,NDL)GO TO 11
      KK=NDL+1
      DO 10 J=KK,NN
      ADLD(J)=ADLD(N)
      10 AMIN(J)=AMIN(N)
      11 NDL=N
      IF(N,1,NUMDV) GO TO 9
      DO 13 N=1,NUMDV
      IF(ADLD(N),1,AMIN(N))ADLD(N)=AMIN(N)
      13 WRITE(IW,102)N,ADLD(N),AMIN(N)
      WRITE(I1) AMIN
      WRITE (I1) ADLD
      RETURN
100  FORMAT(// 35H DESIGN VARIABLE INPUT DATA //
      1 35H DESIGN /
      2 35H VARIABLE INITIAL MIN ALLOWABLE/
      3 35H NUMBER VALUE VALUE //)
101  FORMAT(15,2F10.0)
102  FORMAT(16,2X,2F13.4)
      END
      MAIN7210
      MAIN7220
      MAIN7230
      MAIN7240
      MAIN7250
      MAIN7260
      MAIN7270
      MAIN7280
      MAIN7290
      MAIN7300
      MAIN7310
      MAIN7320
      MAIN7330
      MAIN7340
      MAIN7350
      MAIN7360
      MAIN7370
      MAIN7380
      MAIN7390
      MAIN7400
      MAIN7410
      MAIN7420
      MAIN7430
      MAIN7440
      MAIN7450
      MAIN7460
      MAIN7470
      MAIN7480
      MAIN7490

```

```

SUBROUTINE FLSTIF (APLD,NUMDV,NUMFL,11,12,112)
C*****
C-----FROM ELEMENT STIFFNESS FROM UNIT STIFFNESS MATRICES
C*****
      IMPLCIT REAL*8 (A-H,O-Z)
      REAL*4 APLD,FRC
      DIMENS(ON APLD (NUMDV),S1(24,24),S2(24,24),P1(24,4),P2(24,4)
      COMMON/FM/LM(24),S(24,24,2),P(24,4,2),FM1(1418)
      EQUIVALENCE (S1,S),(S2,S(577)),(P1,P),(P2,P(471))
      BACKSPACE 11
      READ(11) APLD
      REWIND 12
      REWIND 112
      DO 100 M=1,NUMFL
      READ(112) LRD,NH,MV,ND,INVAR,IFX,FPC,(LM(I),I=1,ND),((S(I,J,K),
1 I=1,ND),J=1,ND),K=1,NU),((P(I,J,K),I=1,ND),J=1,4),K=1,NV)
      IF(INVAR.FO.O) GO TO 106
      AREA=APLD(INVAR)*FRC
      XINERT=AREA**IFX
      DO 101 I=1,ND
      DO 102 J=1,4
102 P1(I,J)=P1(I,J)*AREA
      DO 101 J=1,ND
101 S1(I,J)=S1(I,J)*AREA
      IF(NH.FO.O) GO TO 105
      DO 104 I=1,ND
      DO 104 J=1,ND
104 S1(I,J)=S1(I,J)+S2(I,J)*XINERT
105 IF(MV.FO.O) GO TO 106
      DO 107 I=1,ND
      DO 107 J=1,4
107 P1(I,J)=P1(I,J)+P2(I,J)
106 LRD=ND*(ND+4)
      IF(ND.FO.O) GO TO 200
      NN=ND*ND
      CALL PFARAN(S1,S1,24,24,1,ND,ND,1,NN)
      NN=ND*4
      CALL PFARAN(P1,P1,24,4,1,ND,4,1,NN)
200 CALL FLSTFW(LRD,ND,LM,S1,P1,12)
100 CONTINUE
      RETURN
      END

```

```

SUBROUTINE FLSTFW(LRD,ND,LM,S1,P1,12)
C*****
C-----WRITE ELEMENT STIFFNESSES ON TAPE 12
C*****
      IMPLCIT REAL*8 (A-H,O-Z)
      DIMENS(ON LM(NH),S1(ND,ND),P1(ND,4)
      WRITE(12) LRD,ND,LM,S1,P1
      RETURN
      END

```

```

      SUBROUTINE ADDSTF(A,R,STR,NIMF,NBLOCK,NF2R,LL,MRAND,I1,I2,I9,I10,MAINR010
      ) I17)
      C*****
      C-----FORMS GLOBAL EQUILIBRIUM EQUATIONS IN BLOCKS
      C*****
      IMPLICIT REAL*8 (A-H,O-Z)
      REAL*4 STR
      DIMENSION STR(4,LL),A(NF2R,MRAND),R(NF2R,LL)
      COMMON/EM/LM(24),SS(672),FMM(2090)
      NF0R=NF2R/2
      K=NF0R+1
      X=NRLOCK
      MR=DSORT(X)
      MR=MR/2+1
      NFRR=MR*NF2R
      MM=1
      NIMQ=0
      NSHIFT=0
      RFWIND I10
      C*****
      C-----READ STRUCTURE LOAD MULTIPLIERS
      C*****
      RFWIND I1
      READ(I1) STR
      C*****
      C-----FORM EQUATIONS IN BLOCKS (2 BLOCKS AT A TIME)
      C*****
      DO 1000 M=1,NRLOCK,2
      DO 100 I=1,NF2R
      DO 100 J=1,MRAND
      100 A(I,J)=0.
      READ(I17) (R(I,I),I=1,NF0R),L=1,LL)
      IF (M.EQ.NRLOCK) GO TO 200
      READ(I17) (R(I,L),I=K,NF2R),L=1,LL)
      200 RFWIND I9
      RFWIND I2
      MA=I9
      NIMF=NIMQ
      IF (MM,NF,1) GO TO 75
      MA=I2
      NIMF=NIMFL
      NIMQ=0
      75 DO 700 N=1,NIMF
      READ (MA) LPO,ND,(LM(I),I=1,ND),(SS(I),I=1,LPO)
      DO 600 I=1,ND
      LMM=1-LM(I)
      IT=LMM(I)-NSHIFT
      IF (IT,L.F.O.NR,I1,GT,NF2R) GO TO 600
      DO 300 L=1,I1
      DO 200 J=1,4
      KK=ND*(ND+J-1)
      300 R(I,I)=R(I,I)+SS(I+KK)*STR(J,L)
      DO 500 J=J,ND
      J,L=LM(J)+LMM
      IF(J,I) 500,500,390
      390 KK=ND*(J-ND)
      A(I,I,J)=A(I,I,J)+SS(I+KK)
      500 CONTINUE
      600 CONTINUE
      C*****

```

```

C-----DETERMINE IF STIFFNESS IS TO BE PLACED ON UNIT 19          MAINR610
C*****MAINR620
      IF (MM.GT.1) GO TO 700          MAINR630
      DO 650 I=1,ND          MAINR640
      IT=L.M(I) -NSHIFT          MAINR650
      IF (IT.GT.NF2R.AND.IT.LE.NFOR) GO TO 660          MAINR660
650 CONTINUE          MAINR670
      GO TO 700          MAINR680
660 WRITE(19) LRD,ND,(L.M(I),I=1,ND),(SS(I),I=1,LRD)          MAINR690
      NIM9=NIM9+1          MAINR700
700 CONTINUE          MAINR710
      WRITE(J)0 ((A(I,J),I=1,NFOR),J=1,MRAND),((R(I,L),I=1,NFOR),L=1,LI)          MAINR720
      IF (M.FO.NBLOCK) GO TO 1000          MAINR730
      WRITE(J)0 ((A(I,J),I=K,NF2R),J=1,MRAND),((R(I,L),I=K,NF2R),L=1,LI)          MAINR740
      IF (MM.FO.MH) MM=0          MAINR750
      MM=MM+1          MAINR760
1000 NSHIFT=NSHIFT+NF2R          MAINR770
      RETURN          MAINR780
      END          MAINR790

```

```

      SUBROUTINE USOL (MAXR,A,R,NFOR,MH,LL,NBLOCK,NSR,NPRG,NRKS,NT1,          MAINR800
1 NT2,IT)          MAINR810
C*****MAINR820
C-----THIS SUBPROGRAM SOLVES SIMULTANEOUS EQUATIONS FOR DISPLACEMENTS          MAINR830
C-----TAPES USED ARE AS FOLLOW          MAINR840
C-----A AND B (TWO BLOCKS OF STRUCTURAL STIFFNESS AND LOAD VECTORS) ARE          MAINR850
C      STORED ON TAPE NORG          MAINR860
C-----SCRATCH ON NRKS , NT1 , NT2          MAINR870
C-----RESULTS ARE ON TAPE NT2          MAINR880
C*****MAINR890
      IMPLICIT REAL*8 (A-H,O-Z)          MAINR900
      DIMENSION A(NSR),R(NSR),MAXR(NFOR)          MAINR910
      MC=MR+1          MAINR920
      NNR=(MR-1)/NFOR+1          MAINR930
      INC=NFOR-1          MAINR940
      NMR=NFOR*MR          MAINR950
      N2=NT2          MAINR960
      N1=NT1          MAINR970
      REWIND NORG          MAINR980
      REWIND NRKS          MAINR990
C*****MAINR9000
C-----REDUCE EQUATIONS BLOCK-BY-BLOCK          MAINR9010
C*****MAINR9020
      DO 900 N=1,NBLOCK          MAINR9030
      IF (N.GT.1.AND.MH.FO.1) GO TO 110          MAINR9040
      IF (NR.FO.1) GO TO 105          MAINR9050
      REWIND N1          MAINR9060
      REWIND N2          MAINR9070
105 N1=N1          MAINR9080
      IF (N.FO.1) N1=NRG          MAINR9090
      READ (N1) A          MAINR9100
110 DO 300 I=1,NFOR          MAINR9110
      D=A(I)          MAINR9120
      IF (D) 115,300,120          MAINR9130
115 M=NFOR*(N-1)+1          MAINR9140
      WRITE(14,116) M,D          MAINR9150
120 IT=I          MAINR9160

```

```

      DD 125 J=2,M0
      II=II+NEOR
125 A(II)=A(II)/D
      DD 130 J=I,NMR,NEOR
      IF (A(I),NF,D,0) MAXR(I)=J
130 CONTINUE
      JL=I+1
      IF (JL,GT,NEOR) GO TO 300
      II=I
      DD 200 J=JL,NEOR
      II=II+NEOR
      IF (II,GT,NMR) GO TO 200
      C=A(II)
      IF (C,FO,D,0) GO TO 200
      C=C*A(I)
      KK=J-11
      MAX=MAXR(I)
      DD 150 JJ=II,MAX,NEOR
      A(JJ+KK)=A(JJ+KK)-C*A(JJ)
      KK=J+NMR
      JJ=J+NMR
      DD 175 L=1,LL
      A(KK)=A(KK)-C*A(JJ)
      KK=KK+NEOR
175 JJ=J+NEOR
200 CONTINUE
300 CONTINUE
      WRITE (NHKS) A,MAXR
C*****
C-----SURSTJUTIF INTD REMAJNMG FOUATJOMS
C*****
      DD 800 MN=1,NBR
      IF (M+NM,GT,NRI,OCK) GO TO 800
      NI=M1
      IF (M,FO,I) NI=NIOR1
      IF (MN,FO,NRP) NI=NIORG
      READ (N1) R
      II=1+MN*NEOR*NEOR
      DD 700 I=1,NEOR
      II=II
      DD 600 K=1,NEOR
      IF (II,GT,NMR) GO TO 600
      C=A(II)
      IF (C,FO,D,0) GO TO 600
      C=C*A(K)
      MAX=MAXR(K)
      KK=I-11
      DD 640 JJ=II,MAX,NEOR
      R(JJ+KK)=R(JJ+KK)-C*A(JJ)
      KK=J+NMR
      JJ=K+NMR
      DD 650 L=1,LL
      R(KK)=R(KK)-C*A(JJ)
      KK=KK+NEOR
650 JJ=J+NEOR
600 II=II-1MC
700 II=II+NEOR
      IF (NRP,NF,1) GO TO 750
      DD 740 I=1,MKS
740 A(I)=R(II)

```

```

MA1N9170
MA1N9180
MA1N9190
MA1N9200
MA1N9210
MA1N9220
MA1N9230
MA1N9240
MA1N9250
MA1N9260
MA1N9270
MA1N9280
MA1N9290
MA1N9300
MA1N9310
MA1N9320
MA1N9330
MA1N9340
MA1N9350
MA1N9360
MA1N9370
MA1N9380
MA1N9390
MA1N9400
MA1N9410
MA1N9420
MA1N9430
MA1N9440
MA1N9450
MA1N9460
MA1N9470
MA1N9480
MA1N9490
MA1N9500
MA1N9510
MA1N9520
MA1N9530
MA1N9540
MA1N9550
MA1N9560
MA1N9570
MA1N9580
MA1N9590
MA1N9600
MA1N9610
MA1N9620
MA1N9630
MA1N9640
MA1N9650
MA1N9660
MA1N9670
MA1N9680
MA1N9690
MA1N9700
MA1N9710
MA1N9720
MA1N9730
MA1N9740
MA1N9750
MA1N9760

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```

      GO TO 800
750  WRITF (N2) R
800  CONTINUE
      M=M1
      N1=N2
      900 N2=M
C*****
C-----BACKSUBSTITUTION - RESIDUALS ON TAPE N12
C*****
      LS=1, NFOR
      NFR=NFOR*(NHR+1)
      MIM=NHR*NFOR
      MAX=NFR+1,1
      DO 905 J=1, MAX
905  R(J)=0.
      RFWIND NT2
      DO 1000 N=1, NHR, 100
      BACKSPACE NRKS
      READ (NRKS) A, MAXR
      BACKSPACE NRKS
      DO 910 I=1, 1,1
      K=L*NFR
      DO 910 J=1, MIM
      J=K-NFOR
      R(K)=R(I)
910  K=K-1
      J=NMR
      DO 920 I=1, 1,1
      K=(I-1)*NFR
      DO 920 J=1, NFOR
      J=J+1
      K=K+1
920  R(K)=A(I)
      DO 955 J=1, NFOR
      J=NFOR+1-J
      MAX=MAXR(I)
      IF (A(I), F0, 0.) GO TO 955
      DO 950 I=1, 1,1
      KK=J+(I-1)*NFR
      J=KK+1
      JJ=J+NFOR
      C=R(KK)
      DO 940 II=1, MAX, NFOR
      C=C-A(JI)*R(JJ)
940  JJ=JJ+1
950  R(KK)=C
955  CONTINUE
      I=0
      DO 960 I=1, 1,1
      K=(I-1)*NFR
      DO 960 J=1, NFOR
      K=K+1
      J=J+1
960  A(J)=R(K)
      WRITF(NT2) (A(I), I=1, LS)
1000 CONTINUE
      RETURN
116  FORMAT (3H0SET OF EQUATIONS MAY BE SINGULAR /
      , 2H DIAGONAL TERM OF EQUATION ,1S, BE EQUALS ,1F12.4)
      END

```

```

MA1N9770
MA1N9780
MA1N9790
MA1N9800
MA1N9810
MA1N9820
MA1N9830
MA1N9840
MA1N9850
MA1N9860
MA1N9870
MA1N9880
MA1N9890
MA1N9900
MA1N9910
MA1N9920
MA1N9930
MA1N9940
MA1N9950
MA1N9960
MA1N9970
MA1N9980
MA1N9990
MA1N0000
MA1N0010
MA1N0020
MA1N0030
MA1N0040
MA1N0050
MA1N0060
MA1N0070
MA1N0080
MA1N0090
MA1N0100
MA1N0110
MA1N0120
MA1N0130
MA1N0140
MA1N0150
MA1N0160
MA1N0170
MA1N0180
MA1N0190
MA1N0200
MA1N0210
MA1N0220
MA1N0230
MA1N0240
MA1N0250
MA1N0260
MA1N0270
MA1N0280
MA1N0290
MA1N0300
MA1N0310
MA1N0320
MA1N0330
MA1N0340
MA1N0350
MA1N0360

```



```

      SUBROUTINE PRINTD(ID,D,R,NFOR,NIUMNP,LL,NBLOCK,NFO,I2,I8,I4,KODE, MAIN0370
1 KPRINT) MAIN0380
C***** MAIN0390
C-----PRINT NODAL DISPLACEMENTS OR BUCKLING MODE SHAPES MAIN0400
C***** MAIN0410
      IMPLICIT REAL*8 (A-H,O-Z) MAIN0420
      REAL*4 D MAIN0430
      DIMENSION ID(NIUMNP,6),D(6,LL),R(NFOR,LL) MAIN0440
      REAL*4 R MAIN0450
      READ (J8) ID MAIN0460
      IF(KPRINT.FO.O) RETURN MAIN0470
      GO TO (1,2),KODE MAIN0480
1 WRITE(IW,2003) MAIN0490
   GO TO 2 MAIN0500
2 WRITE(IW,2005) MAIN0510
3 READ(J2) MAIN0520
   M=NFO MAIN0530
   NN=NFOR*NBLOCK MAIN0540
   N=NIUMNP MAIN0550
   DO 500 KK=1,NIUMNP MAIN0560
     I=6 MAIN0570
     DO 250 JI=1,6 MAIN0580
       DO 100 L=1,LL MAIN0590
100 D(I,L)=0. MAIN0600
        IF(M.GT.NN) GO TO 150 MAIN0610
        IF (M.EQ.O) GO TO 150 MAIN0620
        READ(I2) R MAIN0630
        NN=NN-NFOR MAIN0640
150 IF(ID(N,I),I,I) GO TO 250 MAIN0650
        K=M-NN MAIN0660
        M=M-1 MAIN0670
        DO 200 L=1,LL MAIN0680
200 D(I,L)=R(K,L) MAIN0690
250 I=I-1 MAIN0700
        WRITE(IW,2004) M,(L,(D(I,L),I=1,6),L=1,LL) MAIN0710
500 M=N-1 MAIN0720
      RETURN MAIN0730
2003 FORMAT (34H NODAL DISPLACEMENTS AND ROTATIONS//
1 5H NODF ,5H LOAD ,11X ,1HX ,11X ,1HY ,11X ,1HZ ,10X ,2HXX,
2 10X ,2HYY ,10X ,2H77/ 5H NO. , 5H CASE /) MAIN0750
2004 FORMAT (1H ,I4,I5,1P3F12.3,3F12.4/(110,3F12.3,3F12.4)) MAIN0770
2005 FORMAT(//21H BUCKLING MODE SHAPES //
1 5H NODF ,5H NODF ,11X ,1HX ,11X ,1HY ,11X ,1HZ ,10X ,2HXX,
2 10X ,2HYY ,10X ,2H77/ 5H NO. , 5H SHAPE /) MAIN0780
      END MAIN0810

```

```

      SUBROUTINE STRESS(ADLD,ASTR,LOAD,STR,R,D,LL,LR,NEQ,NUMDV,MFOR , MAIN0820
1  A,MTOT,LBUCK,INDEF,IDESN,I1,I2,I3,I4,I11,IW) MAIN0830
C*****MAIN0840
C-----CALCULATE STRESSES MAIN0850
C*****MAIN0860
      DIMENS(ON STR(4,LL),D(MFOR,LR),LOAD(NUMDV),ADLD(NUMDV),ASTR(NUMDV) MAIN0870
1  ,A(MTOT) MAIN0880
      REAL*8 R(MFOR,LL) MAIN0890
      COMMON /ELPAR/ NPAR(14),NUMNP,MRAND,NFLYP,N1,N2,N3,N4,N5,MTT, MAIN0900
1  FLIP(9),NRLQCK MAIN0910
      COMMON/BUCK/LT,LH,UM(366) MAIN0920
      REWIND 11 MAIN0930
      READ(11) STR MAIN0940
      READ(11) ASTR MAIN0950
      READ(11) ADLD MAIN0960
      IF(LBUCK.EQ.0) GO TO 200 MAIN0970
      IF(INDEF.ME.0.AND.IDESN.GT.0) GO TO 200 MAIN0980
      REWIND 111 MAIN0990
      REWIND 13 MAIN1000
200 CONTINUE MAIN1010
C*****MAIN1020
C-----PRINT DESIGN VARIABLE ARRAY FOR CURRENT DESIGN MAIN1030
C*****MAIN1040
      CALL DPRINT (ADLD,NUMDV,IW) MAIN1050
      DO 111 I=1,NUMDV MAIN1060
111 LOAD(I)=0 MAIN1070
      N1=(I1-1)/LR +1 MAIN1080
      LH=0 MAIN1090
      DO 1000 JJ=1,N1 MAIN1100
C*****MAIN1110
C-----MOVE DISPLACEMENTS INTO CORF FOR LR LOAD CONDITIONS MAIN1120
C*****MAIN1130
      CALL MOVFD(R,D,MFOR,NRLQCK,MFOR,LL,LR,LH,LT,I2) MAIN1140
C*****MAIN1150
C-----CALCULATE ELEMENT STRESSES AND PERFORM FULLY STRESSED DESIGN MAIN1160
C FOR LR LOAD CONDITIONS MAIN1170
C*****MAIN1180
      DO 1000 M=1,NFLYP MAIN1190
      READ (JR) NPAR MAIN1200
      MTYPE=NPAR(1) MAIN1210
      NPAR(1)=0 MAIN1220
      CALL FLTYPE (A,MTOT,MTYPE,IW) MAIN1230
1000 CONTINUE MAIN1240
      WRITE(11) ASTR,LOAD MAIN1250
      RETURN MAIN1260
      END MAIN1270

```

```

      SUBROUTINE DPRINT (A,MV,I4)
C*****
C-----PRINT DESIGN VARIABLE ARRAY
C*****
      DIMENSION A(MV)
      WRITE(IW,1006)
      NROW=(MV-1)/10+1
      DO 220 M=1,NROW
        M=(M-1)*10
        ISTART=M+1
        ISTOP=M+10
        IF(ISTOP.GT.MV) ISTOP=MV
      220 WRITE(IW,1007) M,(A(I),I=ISTART,ISTOP)
      RETURN
1006 FORMAT(//2RU VALUES OF DESIGN VARIABLES //
1125H          1          2          3          4          5
          2          6          7          8          9         10 / )
1007 FORMAT(1H ,15,1OF12.4)
      END

```

```

      SUBROUTINE MOVED(R,D,NFOR,NBLOCK,NFO,LL,LB,LH,LT,I2)
C*****
C-----MOVE DISPLACEMENTS INTO CORE FOR LB LOAD CONDITIONS FROM TAPE NT
C*****
      DIMENSION D(NFOR,LB)
      READ* R(NFOR,LL)
      REWIND I2
      LT=LH+1
      LLL=1-LT
      LH=LT+LB-1
      IF(LLH.GT.LLL) LH=LLL
      NO=NFOR*NBLOCK
      DO 200 MN=1,NBLOCK
        READ(I2) R
        N=NFOR
        IF (MN.EQ.1) N=NFO-NO+NFOR
        NO=NO-NFOR
        DO 200 J=1,N
          I=NO+J
          DO 200 L=1,LT,LH
            K=1+L*LT
            D(I,K)=R(I,L)
          200
        RETURN
      END

```

```

SUBROUTINE STRSC (ADLD,STR,N,MFO,NUMDV,LL,LR,NTAG)
C*****
C-----SF1 IIP STRESS MATRIX AND CALCULATE STRESSES
C*****
REAL*8 P1,P2,S11,S12,STP,G1,G
DIMENSION STR(4,LL),D(NFO,LR),ADLD(NUMDV),P1(15,4),P2(15,4),
1 S11(15,24),S12(15,24),G1(24,24)
COMMON/JUNK/LT,LH,L,SG(27),IDVAR,IFX,FRC,ARFA,XINERT,DESFNF(333)
COMMON/FM/NI,NW,NS,ND,LM(24),ST(15,24,2),P(15,4,2),G(24,24,3),
1 MSG(3),FM1(3R1)
COMMON/CONTR/IC(6),IDFSN,IC2(6),LRHCK,IC3(4),INDET,IC4(10)
COMMON/UMITS/ IR,1W,1P,1I,12,13,1R,19,110,111,112,113
EQUIVALENCE (P1,P),(P2,P(6)),(S1,S11),(S1(36),S12),(G1,G)
IF (NTAG.FO.O) GO TO 800
NL=1-1,1+1
DO 300 I=1,NS
SG(I)=O.O
DO 300 J=1,4
300 SG(I)=SG(I)+P(I,J)*STP(J,L)
DO 500 J=1,MN
J1=LM(J)
IF(JJ.FO.O) GO TO 500
DO 400 I=1,NS
400 SG(I)=SG(I)+ST1(I,J)*D(J1,NL)
500 CONTINUE
IF(LRHCK.NF.L) RETURN
IF(IDVAR.FO.O) RETURN
IF(INDET.NF.O.AND.IDFSN.GT.O) RETURN
DO 700 I=1,MN
DO 700 J=1,MN
SS=O.
DO 750 K=1,NG
KK=MSG(K)
750 SS=SS+G(I,J,K)*SG(KK)
G1(I,J)=SS
700 G1(J,I)=SS
MN=MN*MN
IF(MN.NF.24) CALL REFRAN(G1,G1,24,24),MN,MN,1,MN)
CALL FLGSTW(MN,LM,G,13)
RETURN
800 READ(IP) NI,NW,NS,ND,IDVAR,IFX,FRC,(IM(J),J=1,MN),((ST(I,J,K),
1 I=1,NS),J=1,MN),K=1,MN),((IP(I,J,K),I=1,NS),J=1,4),K=1,NW)
IF(IDVAR.FO.O) RETURN
IF(LRHCK.FO.O) GO TO 120
IF (INDET.NF.O.AND.IDFSN.GT.O) GO TO 120
REFAN(111) NG,(IG(I,J,K),J=1,MN),J=1,MN),K=1,NG),(MSG(1),I=1,NG)
120 ARFA= ADLD(IDVAR)*FPC
XINERT=ARFA**IFX
DO 100 I=1,NS
DO 101 J=1,4
101 P1(I,J)=P1(I,J)*ARFA
DO 100 J=1,MN
100 S11(I,J)=S11(I,J)*ARFA
REFAN(IR) NJ,(DESFNF(I),I=1,M1)
IF(M1.FO.O) GO TO 900
DO 104 I=1,NS
DO 104 J=1,MN
104 ST1(I,J)=S11(I,J)+ST2(I,J)*XINERT
900 IF(NW.FO.O) RETURN
DO 105 I=1,NS

```

```

      DO 105 J=1,4                                MAIN2310
105  P1(I,J)=P1(I,J)+P2(I,J)                      MAIN2320
      RETURN                                         MAIN2330
      END                                            MAIN2340

```

```

      SUBROUTINE FLGSTW(MD,LM,G,I3)                MAIN2350
C*****                                           MAIN2360
C-----WRITE GEOMETRIC STIFFNESS MATRIX ON TAPE I3          MAIN2370
C*****                                           MAIN2380
      IMPLCIT REAL*8 (A-H,O-7)                   MAIN2390
      DIMENSION LM(MD),G(MD,MD)                  MAIN2400
      LRD=MD*MD                                    MAIN2410
      WRITE(I3) LRD,MD,LM,G                      MAIN2420
      RETURN                                       MAIN2430
      END                                         MAIN2440

```

```

      SUBROUTINE RANAL (A,AD,MTOT,MTOT2)
C*****
C-----BUCKLING ANALYSIS AND DERIVATIVES
C*****
      DIMENSION A(MTOT)
      REAL*8 AD(MTOT2),S,EV,EVEC,G,EVAL
      COMMON/CONTR/ ICYCL,NCYCL,ISCALF,NSCALF,KONVG,IDFSN,IWTMIN,
1WTMIN,EPSIL,DELT1,DELT2,KPINCH,LAUCK,NVEC,NMODF,LH1,ALPA,INDET
2,KPRINT,OMEGA,CORFFT,SMAX,HMAX,MHICK,SF,JS,KOD,NHOUNO
      COMMON/FIPAR/NPAR(14),NIMNP,MRAND,NFLYP,N1,N2,N3,N4,N5,M111,NFO,
1 NIMFI,NIMDV,M1,M2,M3,LL,LH,NFOR,MHLOCK
      COMMON/UNITS/ IR,IW,IP,I1,I2,I3,I4,I9,I10,I11,I12,I13
      COMMON/JUNK/EV(4),S(4,4),G(4,4),EVEC(4,4),EVAL(4),JUN(256)
      DO 99 J=1,NMODF
100 FVAL(I)=0.0
      NJ=1
      IF(INDET,NE,P,AND,IDFSN,GT,0) GO TO 100
C*****
C-----ASSEMBLE GEOMETRIC STIFFNESS MATRIX
C*****
      IG=110
      IF(INDET,FO,0) GO TO 101
      IG=111
      REWIND IG
101 CONTINUE
      NFOR=NFOR*2
      NIME=NIMFI-NHOUNO
      CALL ADGET(AD(N1),NIME,NHLOCK,NFOR,MRAND,I3,I9,IG)
C*****
C-----FIND THE 7 TRANSPOSE * K * 7
C*****
100 CONTINUE
      900 MM=(NFOR+MRAND-1)/NFOR+1
      MM=MM*NFOR
      M=MRAND+11
      ND2=N1+NFOR*M
      ND3=ND2+(MM*NVEC)
      N4=(ND2+MM*NVEC)*2+1
      IF(N4,GT,MTOT) CALL FRROR(N4-MTOT)
      REWIND 110
      CALL MHLRAN(AD(N1),AD(ND2),AD(ND3),NHLOCK,MRAND,M,NVEC,NFOR,MM,
1 I10,I13,I9,S)
C*****
C-----FIND THE PRODUCT 7 TRANSPOSE * G * 7
C*****
      IF(INDET,NE,0) REWIND IG
      CALL MHLRAN(AD(N1),AD(ND2),AD(ND3),NHLOCK,MRAND,MRAND,NVEC,NFOR,
1 MM,IG,I13,I9,G)
C*****
C-----COMPUTE EIGENVALUES (EV) AND EIGENVECTORS (EVEC)
C*****
      CALL EIGJACS(G,EV,EVEC,NVEC)
C*****
C-----CHECK IF EIGENVALUES HAVE CONVERGED
C*****
      NC=0
      DO 200 I=1,NMODF
      IF(DABS(EV(I)-EVAL(I)),GT,DABS(EV(I)/200))INC=1
      200 CONTINUE
C*****

```

```

      MAIN2450
      MAIN2460
      MAIN2470
      MAIN2480
      MAIN2490
      MAIN2500
      MAIN2510
      MAIN2520
      MAIN2530
      MAIN2540
      MAIN2550
      MAIN2560
      MAIN2570
      MAIN2580
      MAIN2590
      MAIN2600
      MAIN2610
      MAIN2620
      MAIN2630
      MAIN2640
      MAIN2650
      MAIN2660
      MAIN2670
      MAIN2680
      MAIN2690
      MAIN2700
      MAIN2710
      MAIN2720
      MAIN2730
      MAIN2740
      MAIN2750
      MAIN2760
      MAIN2770
      MAIN2780
      MAIN2790
      MAIN2800
      MAIN2810
      MAIN2820
      MAIN2830
      MAIN2840
      MAIN2850
      MAIN2860
      MAIN2870
      MAIN2880
      MAIN2890
      MAIN2900
      MAIN2910
      MAIN2920
      MAIN2930
      MAIN2940
      MAIN2950
      MAIN2960
      MAIN2970
      MAIN2980
      MAIN2990
      MAIN3000
      MAIN3010
      MAIN3020
      MAIN3030
      MAIN3040

```

```

C-----COMPUTE IMPROVED COORDINATE VECTORS 7
C*****
      DO 250 I=1,NVFC
      EVAL(I)=EV(I)
      MD2=N1+NEOR*MRAND)
      MD3=MD2+NEOR*NVFC
      MM=MRAND+ILL
      NN=MRAND+NVFC
      CALL SFTF (AD(N1),AD(N1),AD(MD2),AD(MD3),EVFC,NEOR,NVFC,NBLOCK,
      1 MM,NN,MRAND,I3,I9,I10)
      NSR=NEOR*(MRAND+NVFC)
      N2=N1+NEOR
      MD2=MD2/2+1
      MD3=MD2+NSR
      CALL HSDI( A(N1),AD(MD2),AD(MD3),NEOR,MRAND,NVFC,NBLOCK,NSR,
      1 I3,I13,I9,I2,I4)
C*****
C-----TRANSFER IMPROVED COORDINATE VECTORS (7) FROM IAPF 12 TO IAPF 113
C*****
      REWIND 113
      NN=NEOR*NVFC
      DO 400 N=1,NBLOCK
      BACKSPACE 12
      READ(12) (AD(I),I=1,NN)
      WRITE(113) (AD(I),I=1,NN)
      400 BACKSPACE 12
      IF(NC,NF,0) GO TO 900
C*****
C-----EIGENVALUES HAVE CONVERGED - PRINT THEM
C-----COMPUTE STRUCTURE MODE SHAPES
C*****
      WRITE(JW,100) (EV(I),I=1,NVFC)
      MD2=N1+NEOR*NVFC
      CALL SMODE (AD(N1),AD(MD2),EVFC,NEOR,NVFC,NBLOCK,NMODE,I3,I2)
      IF(KPRINT.F0.0) RETURN
C*****
C-----PRINT MODE SHAPES
C*****
      N2=N1+NI*MNP*6
      N3=N2+6*NMODE
      MD3=MD2/2+1
      READ(18) (A(I),I=1,NI*MDV)
      REWIND 19
      WRITE(19) (A(I),I=1,NI*MDV)
      CALL PRINTD(A(N1),A(N2),AD(MD3),NEOR,NI*MNP,NMODE,NBLOCK,NEO,
      1 I3,I8,I4,2,KPRINT)
      RETURN
100) FORMAT(//25H BLOCKING LOAD PARAMETERS //IX,6F20.5)
      END

```

```

      SUBROUTINE ADGSTF(A,NIMF1,NR1,OCK,NF2R,MRAND,I3,I9,I6)
C*****
C-----ASSUMING GF(MF1R) IS STIFFNESS MATRIX CFS
C*****
      IMPL(I3) RFAI*8 (A-H,N-7)
      DIMENS JON A(NF2R,MRAND)
      COMMON/EM/LM(24),SS(672),FMM(2090)
      NFOR=NF2R/2
      K=NFOR*4
      X=NR1,OCK
      MR=DSORT(X)/2+1
      NFRB=MR*NF2R
      MM=1
      NIMQ=0
      MSHIFT=0
      DO 1000 M=1,NR1,OCK,2
      DO 1000 I=1,NF2R
      DO 1000 J=1,MRAND
1000 A(I,J)=0.
      RFWIMD I9
      RFWIMD I3
      MA=I9
      NIMF=NIMQ
      IF (MM,NF,1) GO TO 75
      MA=I3
      NIMF=NIMF1
      NIMQ=0
75 DO 700 N=1,NIMF
      READ (MA) LRD,ND,(LM(I),I=1,ND),(SS(I),I=1,LRD)
      DO 600 I=1,ND
      LMN=LM(I)
      II=LM(I)-MSHIFT
      IF (II,LF,0,OP,II,GT,NF2R) GO TO 600
      DO 500 J=1,ND
      JJ=LM(I)+LMN
      IF (JJ) 500,500,300
300 KK=ND*J-ND
      A(II,JJ)=A(II,JJ)+SS(I+KK)
500 CONTINUE
600 CONTINUE
C*****
C-----DEFINING IF STIFFNESS IS TO BE PLACED ON UNIT I9
C*****
      IF (MM,GT,1) GO TO 700
      DO 650 I=1,ND
      II=LM(I)-MSHIFT
      IF (II,GT,NF2R,AND,II,LF,NFRB) GO TO 660
650 CONTINUE
      GO TO 700
660 WRITE(I9) LRD,ND,(LM(I),I=1,ND),(SS(I),I=1,LRD)
      NIMQ=NIMQ+1
700 CONTINUE
      WRITE(J6) (A(I,J),I=1,NFOR),J=1,MRAND)
      IF (M,F0,NR1,OCK) GO TO 1000
      WRITE(J6) (A(I,J),I=K,NF2R),J=1,MRAND)
      IF (MM,F0,MR) MM=0
      MM=MM+1
1000 MSHIFT=MSHIFT+NF2R
      RETURN
      END

```

```

MA1N3540
MA1N3550
MA1N3560
MA1N3570
MA1N3580
MA1N3590
MA1N3600
MA1N3610
MA1N3620
MA1N3630
MA1N3640
MA1N3650
MA1N3660
MA1N3670
MA1N3680
MA1N3690
MA1N3700
MA1N3710
MA1N3720
MA1N3730
MA1N3740
MA1N3750
MA1N3760
MA1N3770
MA1N3780
MA1N3790
MA1N3800
MA1N3810
MA1N3820
MA1N3830
MA1N3840
MA1N3850
MA1N3860
MA1N3870
MA1N3880
MA1N3890
MA1N3900
MA1N3910
MA1N3920
MA1N3930
MA1N3940
MA1N3950
MA1N3960
MA1N3970
MA1N3980
MA1N3990
MA1N4000
MA1N4010
MA1N4020
MA1N4030
MA1N4040
MA1N4050
MA1N4060
MA1N4070
MA1N4080
MA1N4090
MA1N4100
MA1N4110
MA1N4120
MA1N4130

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      SUBROUTINE MULHAM(A,R,C,NBLOCK,MRAND,M,NVFC,NFOR,MM,NT,I13,I4,I5)  MAIN4140
C*****MAIN4150
C-----MULTIPLICATION OF THE PRODUCT Z TRANSPOSE * (K OR G) * 7  MAIN4160
C-----M IS NO. OF COLUMNS OF A TO BE READ FROM TAPE NT  MAIN4170
C-----MM IS NO. OF ROWS OF R AND C TO BE KEPT IN CORE FOR  MAIN4180
C  MULTIPLICATION A*B  MAIN4190
C*****MAIN4200
      IMPLICIT REAL*8 (A-H,I-Z)  MAIN4210
      DIMENSION A(NFOR,M),R(MM,NVFC),C(MM,NVFC),D(NVFC,NVFC)  MAIN4220
      M3=MM-NFOR  MAIN4230
      REWIND I4  MAIN4240
      REWIND I13  MAIN4250
C*****MAIN4260
C-----INITIALISE MATRICES B AND C  MAIN4270
C*****MAIN4280
      DO 500 I=1,MM  MAIN4290
      DO 500 J=1,NVFC  MAIN4300
      B(I,J)=0.0  MAIN4310
      500 C(I,J)=0.0  MAIN4320
C*****MAIN4330
C-----MATRIX MULTIPLICATION A*B IN BLOCKS  MAIN4340
C*****MAIN4350
      DO 40 I=1,NBLOCK  MAIN4360
      READ(NT) A  MAIN4370
      IF(I.GT.1) GO TO 70  MAIN4380
      M2=MM/NFOR  MAIN4390
      IF(M2.GT.NBLOCK) M2=NBLOCK  MAIN4400
      DO 20 J=1,M2  MAIN4410
      L2=(J-1)*NFOR  MAIN4420
      20 READ(I13) ((B(I2+1,J),J=1,NFOR),J=1,NVFC)  MAIN4430
      GO TO 100  MAIN4440
      70 M2=M2+1  MAIN4450
C*****MAIN4460
C-----MOVE MATRICES B AND C UP BY ONE BLOCK LENGTH  MAIN4470
C*****MAIN4480
      DO 80 I=1,M3  MAIN4490
      DO 80 J=1,NVFC  MAIN4500
      C(I,J)=C(I+NFOR,J)  MAIN4510
      80 B(I,J)=B(I+NFOR,J)  MAIN4520
      DO 81 I=1,NFOR  MAIN4530
      DO 81 J=1,NVFC  MAIN4540
      C(I+M3,J)=0.0  MAIN4550
      81 B(I+M3,J)=0.0  MAIN4560
      IF(M2.GT.NBLOCK) GO TO 100  MAIN4570
      READ(I13) ((B(I+M3,J),J=1,NFOR),J=1,NVFC)  MAIN4580
      100 CONTINUE  MAIN4590
C*****MAIN4600
C-----MATRIX MULTIPLICATION A*B  MAIN4610
C*****MAIN4620
      DO 120 I3=1,NVFC  MAIN4630
      DO 120 I2=1,NFOR  MAIN4640
      DO 140 I2=1,MRAND  MAIN4650
      I12=I1+I2-1  MAIN4660
      140 C(I1,I3)=C(I1,I3)+A(I1,I2)*B(I12,I3)  MAIN4670
      DO 120 I2=2,MRAND  MAIN4680
      I12=I1+I2-1  MAIN4690
      120 C(I12,I3)=C(I12,I3)+A(I1,I2)*B(I1,I3)  MAIN4700
C*****MAIN4710
C-----WRITE ONE BLOCK OF C ON TAPE I4  MAIN4720
C*****MAIN4730

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      WRITE ( I9 ) ( (C(I,J),I=1,NFOR),J=1,NVFC )
      40 CONTINUE
C-----FORM MATRIX PRODUCT B TRANSPOSE A*B
C-----PESULT IS MATRIX D
C*****
      REWIND I13
      REWIND I9
      DO 550 I=1,NVFC
      DO 550 J=1,NVFC
      D(I,J)=0.0
      DO 200 L=1,NBACK
      READ ( J13 ) (B(I,J),J=1,NFOR),L=1,NVFC)
      READ( I9 ) (C(I,J),I=1,NFOR),J=1,NVFC)
C*****
C-----MATRIX MULTIPLICATION B TRANSPOSE * C
C*****
      DO 250 L1=1,NVFC
      DO 250 L2=1,NFOR
      DO 250 L3=1,NVFC
      250 D(L1,L3)=D(L1,L3)+B(L2,L1)*C(L2,L3)
      200 CONTINUE
      RETURN
      END

```

```

      SUBROUTINE FIGJAC ( S,G,EV,EVFC,NVFC)
      IMPLICIT REAL*8 (A-H,O-Z)
C*****
C-----COMPUTE EIGENVALUES AND NORMALIZED EIGENVECTORS
C*****
      DIMENSION S(NVFC,NVFC),G(NVFC,NVFC),EV(NVFC),EVFC(NVFC,NVFC)
      IF(NVFC.GT.1) GO TO 300
      EV(1)=-S(1,1)/G(1,1)
      G1=-1.0/G(1,1)
      IF(G(1,1).GT.0) G1=-G1
      EVFC(1,1)=DSORT(G1)
      GO TO 400
      300 DFT=G(1,1)*G(2,2)-G(2,1)**2
      G1=S(1,1)*G(2,2)+S(2,2)*G(1,1)-2.0*S(1,2)*G(1,2)
      G2=S(1,1)*S(2,2)-S(1,2)**2
      D=DSORT(G1-G2-4.0*DFT**2)
      EV(1)=(-G1-D)/(2.0*DFT)
      EV(2)=(-G1+D)/(2.0*DFT)
      DO 60 I=1,2
      RAT=(-S(1,1)+EV(I)*G(1,1))/(S(1,2)+EV(I)*G(1,2))
      C1=G(1,1)+2.0*S(1,2)*RAT+G(2,2)*RAT**2
      IF(C1.LT.0.) C1=-C1
      EVFC(1,1)= DSORT( 1.0/C1)
      60 EVFC(2,1)=RAT*EVFC(1,1)
      400 CONTINUE
      RETURN
      END

```

```

      SUBROUTINE SETF (SK,SK1,F,C,FVFC,NFOR,NVFC,NBLOCK,MM,NN,MHAND,I3, MAIN5250
      I 19,110) MAIN5260
C***** MAIN5270
C-----SETUP STRUCTURE STIFFNESS MATRIX AND F=6*7*0 FOR SOLVING MAIN5280
C IMPROVED COORDINATE VECTORS USING SUBROUTINE USOL MAIN5290
C***** MAIN5300
      IMPLICIT REAL*8 (A-H,O-Z) MAIN5310
      DIMENSION SK(NFOR,MM),SK1(NFOR,NN),F(NFOR,NVFC),C(NVFC),
      FVFC(NVFC,NVFC) MAIN5320
      REWIND 13 MAIN5330
      REWIND 19 MAIN5340
      REWIND 110 MAIN5350
      DO 500 N=1,NBLOCK MAIN5360
      READ(110) SK MAIN5370
      READ(110) SK1 MAIN5380
C***** MAIN5390
C-----CALCULATE THE PRODUCT G*7*FVFC MAIN5400
C***** MAIN5410
      READ(19) F MAIN5420
      DO 100 J=1,NFOR MAIN5430
      DO 150 I=1,NVFC MAIN5440
      C(I,J)=0.0 MAIN5450
      DO 150 K=1,NVFC MAIN5460
      150 C(I,J)=C(I,J)+F(1,K)*FVFC(K,J) MAIN5470
      DO 200 J=1,NVFC MAIN5480
      200 F(1,J)=C(I,J) MAIN5490
      100 CONTINUE MAIN5500
      500 WRITE(13) SK1 MAIN5510
      RETURN MAIN5520
      END MAIN5530

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      SUBROUTINE SMODE (7,Y,FVFC,NFOR,NVFC,NBLOCK,NMODE,I3,I2 ) MAIN5540
C***** MAIN5550
C-----CALCULATE MODE SHAPES Y=7*FVFC MAIN5560
C***** MAIN5570
      IMPLICIT REAL*8 (A-H,O-Z) MAIN5580
      DIMENSION Z(NFOR,NVFC),Y(NFOR,NMODE),FVFC(NVFC,NVFC) MAIN5590
      REWIND 13 MAIN5600
      REWIND 12 MAIN5610
      DO 100 N=1,NBLOCK MAIN5620
      READ(12) Z MAIN5630
      DO 200 I=1,NFOR MAIN5640
      DO 200 J=1,NMODE MAIN5650
      C1=0. MAIN5660
      DO 250 K=1,NVFC MAIN5670
      250 C1=C1+Z(I,K)*FVFC(K,J) MAIN5680
      200 Y(I,J)=C1 MAIN5690
      100 WRITE(13) Y MAIN5700
      RETURN MAIN5710
      END MAIN5720

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      SHARDTIME DERV( APLD,0,0,H,L,R1,NBUCK,NFOR,NBLCK,NEQ,NUMDV,NMODE, MAIN5730
      1 IL,NUMFI,11,12,13,112,IR,1W) MAIN5740
C*****MAIN5750
C-----CALCULATE RUCKLING DERIVATIVES MAIN5760
C*****MAIN5770
      IMPLICIT REAL*8 (A-H,O-7) MAIN5780
      REAL*8 D,APLD,0,FRC MAIN5790
      DIMENS(ION N(NFOR,NMODE),S1(24,24),S2(24,24) MAIN5800
      DIMENS(ION D(NEO,LRI),,APLD(NUMDV),0(NUMDV,LRI) MAIN5810
      COMMON /FM/LM(24),S(24,24,2),P(24,4,2),C(4,24),FM1(1322) MAIN5820
      EQUIVALENCE (S1,S),(S2,S(577)) MAIN5830
      READ(1) APLD MAIN5840
      READ (1) APLD MAIN5850
      NT=(NBUCK-1)/LR1+1 MAIN5860
      LR1=LR1 MAIN5870
      LR=0 MAIN5880
      REWMND 12 MAIN5890
      REWMND 13 MAIN5900
      DO 10 I=1,NT MAIN5910
C*****MAIN5920
C-----MOVE RUCKLING MODESHAPES INTO CORE FOR LR1 MODES MAIN5930
C*****MAIN5940
      CALL MOVED (R,D,NFOR,NBLCK,NFO,NMODE,LR1,LH,LI,13) MAIN5950
      DO 50 J=1,LH1 MAIN5960
      DO 50 I=1,NUMDV MAIN5970
      DO 10 I,J)=0,0 MAIN5980
      REWMND 112 MAIN5990
      DO 100 NN=1,NUMFI MAIN6000
C*****MAIN6010
C-----CALCULATE STIFFNESS MATRIX DERIVATIVES MAIN6020
C*****MAIN6030
      READ (112) LR0,NI,NV,ND,I0VAR,IFX,FRC,(LM(I),I=1,ND),(I(S(I,J,K), MAIN6040
      1 I=1,ND),J=1,ND),K=1,NI),(I(P(I,J,K),I=1,ND),J=1,4),K=1,NV) MAIN6050
      IF(I0VAR.F0,0) GO TO 100 MAIN6060
      IF(NI.F0,1) GO TO 501 MAIN6070
      FR=IFX*(FRC*APLD(I0VAR))**(IFX-1) MAIN6080
      DO 502 I=1,ND MAIN6090
      DO 502 J=1,ND MAIN6100
      S1(I,J)=S1(I,J)+S2(I,J)*FR MAIN6110
      502 S1(J,I)=S1(I,J) MAIN6120
      501 DO 300 I=1,ND MAIN6130
      DO 300 J=1,ND MAIN6140
      S1(I,J)=S1(I,J)*FRC MAIN6150
      300 S1(J,I)=S1(I,J) MAIN6160
C*****MAIN6170
C-----CALCULATE THE RUCKLING DERIVATIVES MAIN6180
C*****MAIN6190
      IF(11.F0,NT) I1=NBUCK-(11-1)*LR1 MAIN6200
      DO 950 I=1,I1 MAIN6210
      DO 950 J=1,ND MAIN6220
      C(I,J)=0,0 MAIN6230
      DO 610 K=1,ND MAIN6240
      MM=LM(K) MAIN6250
      IF(MM.F0,0) GO TO 610 MAIN6260
      DO 600 I=1,I1 MAIN6270
      DO 600 J=1,ND MAIN6280
      600 C(I,J)=C(I,J)+D(MM,I)*S1(K,I) MAIN6290
      610 CONTINUE MAIN6300
      DO 700 I=1,ND MAIN6310
      MM=LM(I) MAIN6320

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      IF(MM,LF,0) GO TO 700
      DO 710 K=1,L1
710  D( IDVAR,K)=D( IDVAR,K)+C(K,I)*D(MM,K)
700  CONTINUE
100  CONTINUE
      DO 850 ,I=1,L1
850  WRITE(12) (D(K,I),K=1,NUMDV)
10  CONTINUE
      RETURN
      END

```

```

MAIN6330
MAIN6340
MAIN6350
MAIN6360
MAIN6370
MAIN6380
MAIN6390
MAIN6400
MAIN6410
MAIN6420

```

```

      SUBROUTINE DESIGN( AOLD,ASTR,LOAD,WT,STR,NUMDV,L,IU)
      C*****
      C-----EVALUATE THE CURRENT DESIGN AND PERFORM REDESIGN OPERATION
      C*****
      REAL*8 FV
      DIMENSION AOLD(NUMDV),ASTR(NUMDV),LOAD(NUMDV),WT(NUMDV),STR(4,11)
      COMMON/CONTRO/ICYCL,NCYCL,ISCALE,NSCALE,KSCALE,KONVG,INFSN,IWTMIN,
      IWTIM,FPSIL,DELTA,DELTA2,KPINCH,LRUCK,NVFC,NMDF,LH1,ALPA,INDEF
      2,KPRINT,CONST,COFFET,SMAX,UMAX,NRUCK,SF,JS,KONF,NROUNO
      COMMON/HUNK/FV(4),ORAT(4),FVP(4),JUN(252)
      COMMON/HUNITS/IR,IW,IP,I1,I2,I3,I4,I5,I10,I11,I12, ,I13
      REWIND IJ
      READ ( IJ ) STR
      READ ( IJ ) AOLD
      READ ( IJ ) AOLD
      READ ( IJ ) ASTR ,LOAD
      READ ( IJ ) WT
      KONVG=1
      SMAX=0.
      SMIN=1.0E20
      UMAX=0.
      NRUCK=0
      JS=0
      KONF=0
      WT=0.
      RCONST=2.0
      DO 22 I=1,NUMDV
      22 WT=WT+AOLD(I)*IWT(I)
      C*****
      C-----COMPUTE MAX. AND MIN. STRESS RATIOS AND PRINT THEM
      C*****
      DO 68 I=1,NUMDV
      P=ASTR(I)/AOLD(I)
      IF(R,L,F,SMAX) GO TO 69
      SMAX=P
      TMAX=I
      LMAX=LOAD(I)
      69 IF(R,G,F,SMIN) GO TO 68
      SMIN=R
      TMIN=I
      LMIN=LOAD(I)
      68 CONTINUE
      WRITE( IW,1000)INFSN,SMAX,LMAX,TMAX,SMIN,LMIN,TMIN
      IF(LRUCK.FO.O) GO TO 80
      C*****
      C-----PRINT BUCKLING LOAD RATIOS
      C*****
      WRITE( IW,2002)
      DO 70 I=1,NMDF
      ORAT(I)=COFFET/FV(I)
      70 WRITE( IW,2001) ORAT(I),LRUCK
      C*****
      C-----CALCULATE NO. OF POSSIBLE ACTIVE BUCKLING CONSTRAINTS
      C*****
      DO 70 I=1,NMDF
      ARD=ORAT(I)
      IF(ARD,L,F,O.) GO TO 71
      IF(KSCALE,F,G,O) ARD=(ARD)**(1.0/KSCALE)
      P=ARD/SMAX
      IF(P,I,CONST) GO TO 71

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      NRIICK=NRIICK+1
      IF(ARD.GT.DMAX) DMAX=ARD
      FVP(NRIICK)=FV(I)
      GO TO 70
71) DRAT(I)=0.
70) CONTINUE
80) SF=SMAX
      IF(SF.LT.DMAX) SF=DMAX
      IF(SF.LT.DELTA1.DD.SF.GT.DELTA2) GO TO 305
      IF(SMIN.LT.DELTA1.DR.SMIN.GT.DELTA2) GO TO 83
      WRITE(IW,1004)
      KONVIG=4
      WRITE(IW,1008) WT
      GO TO 85
83) WRITE(IW,1003)
      WRITE(IW,1008) WT
84) WRITE(IW,2005)
      KONVIG=2
      IF(DMAX.GT.SMAX) KONVIG=2
      ISCALE=0
      ICYCL=ICYCL+1
      IF(ICYCL.LE.NCYCL) GO TO 86
      KONVIG=4
      WRITE(IW,1005) NCYCL
      GO TO 85
305) IF(KSCALE.GE.0.AND.SF.LT.RCONST) GO TO 101
      WRITE(IW,1002)
      KOFF=1
      GO TO 84
101) IF(KSCALE.EQ.0) GO TO 803
      IS=1
      DO 103 I=1,NUMDV
103) AOLD(I)=AOLD(I)*SF
      WRITE(IW,2004)
      CALL MFSG(SMAX,DMAX,DMAX,IW)
      WRITE(IW,2006)
      CALL DPRINT(AOLD,NUMDV,IW)
      WT=WT*SF
      WRITE(IW,1008) WT
      GO TO 84
803) WRITE(IW,1002)
      WRITE(IW,2004)
      SF=SMAX
      ISCALE=ISCALE+1
      IF(ISCALE.GT.NSCALE) GO TO 203
      IF(NRIICK.NE.0) RETURN
      DO 239 I=1,NUMDV
239) AOLD(I)=AOLD(I)*SF
      PFWIND I
      READ(I) STP
      READ(I) ASTP
      WRITE(I) AOLD
      CALL MFSG(SMAX,SMAX,SMAX,IW)
      RETURN
203) KONVIG=4
      WRITE(IW,1001) NSCALE
      GO TO 85
86) IF(KOFF.EQ.1) GO TO 803
      IF(WT.LT.MIN) GO TO 802
      R=(WT-MIN)/MAX

```

```

      MA1N7030
      MA1N7040
      MA1N7050
      MA1N7060
      MA1N7070
      MA1N7080
      MA1N7090
      MA1N7100
      MA1N7110
      MA1N7120
      MA1N7130
      MA1N7140
      MA1N7150
      MA1N7160
      MA1N7170
      MA1N7180
      MA1N7190
      MA1N7200
      MA1N7210
      MA1N7220
      MA1N7230
      MA1N7240
      MA1N7250
      MA1N7260
      MA1N7270
      MA1N7280
      MA1N7290
      MA1N7300
      MA1N7310
      MA1N7320
      MA1N7330
      MA1N7340
      MA1N7350
      MA1N7360
      MA1N7370
      MA1N7380
      MA1N7390
      MA1N7400
      MA1N7410
      MA1N7420
      MA1N7430
      MA1N7440
      MA1N7450
      MA1N7460
      MA1N7470
      MA1N7480
      MA1N7490
      MA1N7500
      MA1N7510
      MA1N7520
      MA1N7530
      MA1N7540
      MA1N7550
      MA1N7560
      MA1N7570
      MA1N7580
      MA1N7590
      MA1N7600
      MA1N7610
      MA1N7620

```

```

      IF(R.IT,FPSI) GO TO 503
      KDMVG=4
      WRITE(IW,1000) IWTMIN
      GO TO 85
502  IWTMIN=WT
      IWTMIN=IDFSN
503  IF(NBUCK.NF.0) RETURN
      REWIND II
      READ (II) STP
      READ (II) ANLD
      WRITE(II) ASTR
      RETURN
85  IF(KPUNCH.F0.0) RETURN
      REWIND II
      READ (II) STP
      READ (II) ASTP
      DO 250 I=1,NUMDV
250  WRITE(JP,1010) I,ANLD(I),ASTR(I)
      RETURN
1000 FORMAT (// 32H *****/
1      28H EVALUATION OF DESIGN NUMBER,I4 /
2      32H *****//
350H      STRESS RATIO    LOAD COND    DES VARIABLE/,
4  4H MAX,F1R,4,I10,I13, /
5  4H MIN,F1R,4,I10,I13 /)
1001 FORMAT(49H TERMINAL DESIGN---NUMBER OF SCALING OPERATIONS= ,I4//)
1002 FORMAT(//23H DESIGN IS NOT CRITICAL//)
1003 FORMAT(//23H DESIGN IS CRITICAL //)
1004 FORMAT(//23H DESIGN IS ACCEPTABLE //)
1005 FORMAT(//48H TERMINAL DESIGN---NUMBER OF CRITICAL DESIGNS =,I5//)
1006 FORMAT(//19H STRUCTURAL WEIGHT=,F11.4)
1009  FORMAT(60H TERMINAL DESIGN---HIGHEST CRITICAL DESIGN IS DESIGN
      1NUMBER,I4//)
1010 FORMAT(15.2F10.5)
2001 FORMAT(7X,F13.4,2I10)
2002 FORMAT(48H      MAX BUCK PATIENS    LOAD COND    / )
2004 FORMAT(//1X,33HUNIFORM SCALING OPERATION FOLLOWS)
2005 FORMAT(//1X,26HREDDESIGN OPERATION FOLLOWS)
2006 FORMAT(//1X,48HDESIGN VARIABLES OF SCALED (CRITICAL) DESIGN ARE)
      END

```

```

MAIN7640
MAIN7640
MAIN7650
MAIN7660
MAIN7670
MAIN7680
MAIN7690
MAIN7700
MAIN7710
MAIN7720
MAIN7730
MAIN7740
MAIN7750
MAIN7760
MAIN7770
MAIN7780
MAIN7790
MAIN7800
MAIN7810
MAIN7820
MAIN7830
MAIN7840
MAIN7850
MAIN7860
MAIN7870
MAIN7880
MAIN7890
MAIN7900
MAIN7910
MAIN7920
MAIN7930
MAIN7940
MAIN7950
MAIN7960
MAIN7970
MAIN7980
MAIN7990
MAIN8000
MAIN8010
MAIN8020

```



```

SUBROUTINE RDEFSN (ACLD,ORI,I0),ASTR,ARHC,IWT,OPTIN,STR,NUMDV,IU, MAINR030
1 I1,NBUCK) MAINR040
C***** MAINR050
C-----BUCKLING CONSTRAINT REDESIGN OR SCALING MAINR060
C***** MAINR070
REAL*P FV MAINR080
DIMENS ION I0)(NUMDV),ACLD(NUMDV),ASTR(NUMDV),ARHC(NUMDV),
IORT(NUMDV,NBUCK),IWT(NUMDV),STR(4,LI),OPTIN(NUMDV) MAINR090
COMMON/CONTR/ ICYCL,NCYCL,ISCALE,KSCALE,KONVG,INFSN,IWTMIN,MAINR110
IWTMIN,FPSIL,DELTAI,DELT2,KPINCH,LBUCK,NVFC,NMODF,LH1,ALPA,INDEF MAINR120
2,KPRINT,CONST,CPEFFT,SMAX,DMAX,NBUCKK,SF,IS,KONF,NBMINI MAINR130
COMMON/JUNK/FV(4),ORAT(4),FVP(4),R(4,4),R(4),AMDA(4),S(4),JUN(324) MAINR140
COMMON/UNIT/IR,IW,IP,I1,I2,I3,I4,I5,I6,I7,I8,I9,I10,I11,I12,I13
DATA TAG1,TAG2/3HACT,4HPASS/ MAINR160
DEFL5=5.0*(1.0-DEFLTA1) MAINR170
DEFL11=1.0-DEFL5 MAINR180
DEFL122=1.0+DEFL5 MAINR190
BACKSPACE IU MAINR200
READ(IU) IWT MA1NR210
REWIND IU MA1NR220
READ(I1) STR MA1NR230
READ(I1) ACLD MA1NR240
READ(I1)ACLD MA1NR250
REWIND IU MA1NR260
DO 61 I=1,NBUCK MA1NR270
61 READ(I2) (ORI(I,I),I=1,NUMDV) MA1NR280
DO IF(KONVG,FO,1) GO TO 50 MA1NR290
C***** MA1NR300
C-----BUCKLING CONSTRAINT REDESIGN MA1NR310
C***** MA1NR320
READ(I1) ASTR MA1NR330
REWIND IU MA1NR340
IF(S,NF,1) GO TO 101 MA1NR350
C***** MA1NR360
C-----CONVEPT BUCKLING RATIOS AND DERIVATIVES TO THE SCALED DESIGN MA1NR370
C***** MA1NR380
DO 99 I=1,NUMDV MA1NR390
99 ACLO(I)=ACLO(I)*SF MA1NR400
SFF=SF*KSCALE MA1NR410
SFFF=SF*(KSCALE-1) MA1NR420
DO 102 I=1,NBUCK MA1NR430
FVP(I)=FVP(I)*SFF MA1NR440
DO 102 J=1,NUMDV MA1NR450
102 ORI(I,J)=ORI(I,I)*SFFF MA1NR460
C***** MA1NR470
C-----CLASSIFY DESIGN VARIABLES EITHER AS ACTIVE OR PASSIVE MA1NR480
C-----DESIGN VARIABLES WITH THEIR DERIVATIVES FOR ALL POTENTIALLY ACTIVE MA1NR490
C BUCKLING MODES AS POSITIVE OR PASSIVE VARIABLES MA1NR500
C***** MA1NR510
101 DO 50 I=1,NUMDV MA1NR520
DO 51 J=1,NBUCK MA1NR530
IF(ORI(I,J).GT,0.) GO TO 49 MA1NR540
51 CONTINUE MA1NR550
I0(I)=0 MA1NR560
GO TO 50 MA1NR570
49 I0(I)=1 MA1NR580
50 CONTINUE MA1NR590
C***** MA1NR600
C-----ITERATION TO FIND OUT ACTIVE/PASSIVE CLASSIFICATION OF DESIGN MA1NR610
C VARIABLES MA1NR620

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C*****MA1NR630
700 MAC=] MA1NR640
C*****MA1NR650
C-----CHECK IF ALL DESIGN VARIABLES ARE PASSIVE MA1NR660
C*****MA1NR670
      DO 52 I=1,NUMDV MA1NR680
      IF (ID(I),FO,1) GO TO 599 MA1NR690
      52 CONTINUE MA1NR700
C*****MA1NR710
C-----ALL DESIGN VARIABLES ARE PASSIVE. MAKE THEM ACTIVE FOR NEXT CYCLE MA1NR720
C*****MA1NR730
      ICON=0 MA1NR740
      GO TO 602 MA1NR750
C*****MA1NR760
C-----CALCULATE RIGHT HAND SIDES OF SIMULTANEOUS EQUATIONS FOR LAMBDAS MA1NR770
C*****MA1NR780
      599 DO 80 I=1,4 MA1NR790
      R(I)=0. MA1NR800
      AMDA(I)=0. MA1NR810
      DO 80 J=1,4 MA1NR820
      DO 100 J=1,NUMDV MA1NR830
      DO 100 I=1,NUMDV MA1NR840
      DP=0.0 MA1NR850
      DA=0.0 MA1NR860
      DO 110 J=1,NUMDV MA1NR870
      TF(ID(I,J),FO,1) GO TO 120 MA1NR880
      DP=DP+DR(I,J,1)*(ASTR(J,1)-APLD(J)) MA1NR890
      GO TO 110 MA1NR900
      120 DA=DA+DR(I,J,1)*APLD(J) MA1NR910
      110 CONTINUE MA1NR920
      100 R(I)=(-ALPA)*DA+COEFFT-FVP(I)-DP MA1NR930
C*****MA1NR940
C-----DEVELOP COEFFICIENT MATRIX FOR LAMBDAS MA1NR950
C*****MA1NR960
      DO 250 I=1,NUMDV MA1NR970
      DO 250 J=1,NUMDV MA1NR980
      DO 260 K=1,NUMDV MA1NR990
      TF(ID(K),FO,1) D(I,J)=D(I,J)+DR(I,K,1)*OR(I,K,1)*APLD(K)/UWT(K) MA1N9000
      260 CONTINUE MA1N9010
      D(I,J)= D(I,J)*(-ALPA) MA1N9020
      250 D(J,I)=D(I,J) MA1N9030
C*****MA1N9040
C-----ITERATION TO FIND ACTIVE BUCKLING CONSTRAINTS (IF +VE LAMBDA'S) MA1N9050
C*****MA1N9060
      CALL DISP(R,D,AMDA,NUMDV,ICON) MA1N9070
      TF(ICON,NE,0) GO TO 601 MA1N9080
      602 WRITE(14,1006) ICON MA1N9090
      REWIND 11 MA1N9100
      READ(11) STP MA1N9110
      READ(11) APLO MA1N9120
      WRITE(11) ASTR MA1N9130
      RETURN MA1N9140
C*****MA1N9150
C-----CALCULATE DEFLECTION FROM BUCKLING, CONSTRAINTS MA1N9160
C*****MA1N9170
      601 DO 500 I=1,NUMDV MA1N9180
      C=0. MA1N9190
      DO 510 J=1,NUMDV MA1N9200
      510 C=C+AMDA(J)*OR(I,J,1) MA1N9210
      OPTIM(I)=C/UWT(I) MA1N9220

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```

C=(ALPHA+(1.0-ALPHA)*OPTIM(I))*ADL(I)
IF(C.GF.ASTR(I)) GO TO 520
IF(ID(I).EQ.1) MAC=0
ID(I)=0
ARHC(I)=ASTR(I)
GO TO 500
520 ARHC(I)=C
IF(ID(I).EQ.0) MAC=0
ID(I)=1
500 CONTINUE
C*****
C-----CHECK FOR ANY CHANGE IN ACTIVE/PASSIVE CLASSIFICATION OF DESIGN
C    VARIABLES
C*****
IF(MAC.EQ.0) GO TO 700
C*****
C-----PRINT OPTIMALITY INDEX
C*****
WRITE(IW,2002)
DO 750 I=1,NIMDV
TAG=TAG1
IF(ID(I).EQ.0) TAG=TAG2
750 WRITE(IW,2003) I,TAG,OPTIM(I)
WRITE(IW,1006) ICON
IF(SF.GT.DFLT22.OR.DP.SF.LT.DFLT1A1) GO TO 701
IF(DMAX.GT.DFLT22.OR.DMAX.LT.DFLT1A1) GO TO 701
C*****
C-----CHECK FOR BUCKLING DESIGN CONVERGENCE.
C*****
DO 702 I=1,NIMDV
IF(ID(I).EQ.0) GO TO 702
C=OPTIM(I)
IF(C.GT.DFLT22.OR.C.LT.DFLT11) GO TO 701
702 CONTINUE
KONVG=4
WRITE(IW,2001)
RETURN
C*****
C-----CALCULATE SCALE FACTOR FOR UNIFORM SCALING FROM BUCK. CONSTRAINTS
C*****
501 DO 503 J=1, NBUCK
S(J)=0
DO 502 I=1,NIMDV
502 S(J)=S(J)+ORI(I,J)*ADL(I)
503 S(J)=(COEFF1-EVP(J))/S(J)+1.0
DE=0.
DO 504 I=1,NBUCK
IF(S(I).GT.DE)DE=S(I)
504 CONTINUE
CALL MESG(SF,DE,DE,IW)
IF(DE.GT.SF)SF=DE
DO 505 I=1,NIMDV
505 ARHC(I)=ADL(I)*SF
701 REWIND J
PEAD(I) STP
PEAD(J) ADL
WRITE(I) ARHC
RETURN
1006 FORMAT(1X,42HP, OF ACTIVE BUCKLING CONSTRAINTS ARE ,15)
2001 FORMAT(42H BUCKLING - CRITICAL DESIGN HAS CONVERGED //)

```

```

2002 FORMAT(//A2H OPTIMALITY INDEX OF DESIGN VARIABLES FOR BUCKLING CONTAIN9830
1STPAJN1S //5X,5HNDV ND,1X,7HAC1/PAS,4X,5HINDEX /)
2003 FORMAT(5X,15,A10,F15.5)
END

```

```

MAIN9840
MAIN9850
MAIN9860

```

```

SUBROUTINE DISP(R,N,AMDA,NDISP,ICCN)
C*****
C-----FIND OUT ACTIVE LAMBDA'S IF +VF DISP. CONSTRAINTS)
C*****
DIMENSION R(4),D(4,4),AMDA(4)
ICCN=NDISP
IF(NDISP.GT.1) GO TO 900
AMDA(1)=R(1)/D(1,1)
IF(AMDA(1).GT.0.) RETURN
AMDA(1)=0.
ICCN=0
RETURN
900 IF(D(2,2).NE.0.) GO TO 10
IF(D(1,1).NE.0.) GO TO 11
AMDA(1)=0.
AMDA(2)=0.
ICCN=0
RETURN
11 AMDA(1)=P(1)/D(1,1)
AMDA(2)=0.
ICCN=1
IF(AMDA(1).GT.0.) RETURN
AMDA(1)=0.
ICCN=0
RETURN
10 IF(D(1,1).NE.0.) GO TO 20
AMDA(1)=0.
AMDA(2)=R(2)/D(2,2)
ICCN=1
IF(AMDA(2).GT.0.) RETURN
AMDA(2)=0.
ICCN=0
RETURN
20 DFI=D(1,1)*D(2,2)-D(1,2)*D(1,2)
C1=D(1,1)*D(2,2)*1.0E-06
IF(ABS(DFI).GT.C1) GO TO 30
A1=R(1)/D(1,1)
A2=R(2)/D(2,2)
IF(A1.LE.0..AND.A2.LE.0.) GO TO 40
IF(A2.GT.A1) GO TO 50
AMDA(1)=A1
AMDA(2)=0.
ICCN=1
RETURN
50 AMDA(1)=0.
AMDA(2)=A2
ICCN=1
RETURN
40 AMDA(1)=0.
AMDA(2)=0.
ICCN=0

```

```

MAIN9870
MAIN9880
MAIN9890
MAIN9900
MAIN9910
MAIN9920
MAIN9930
MAIN9940
MAIN9950
MAIN9960
MAIN9970
MAIN9980
MAIN9990
MAIN0000
MAIN0010
MAIN0020
MAIN0030
MAIN0040
MAIN0050
MAIN0060
MAIN0070
MAIN0080
MAIN0090
MAIN0100
MAIN0110
MAIN0120
MAIN0130
MAIN0140
MAIN0150
MAIN0160
MAIN0170
MAIN0180
MAIN0190
MAIN0200
MAIN0210
MAIN0220
MAIN0230
MAIN0240
MAIN0250
MAIN0260
MAIN0270
MAIN0280
MAIN0290
MAIN0300
MAIN0310
MAIN0320
MAIN0330
MAIN0340
MAIN0350
MAIN0360
MAIN0370

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```

      RETURN
30  AMDA(1)=(D(2,2)*R(1)-D(1,2)*R(2))/DFL
    AMDA(2)=(-D(2,1)*R(1)+D(1,1)*R(2))/DFL
    IF (AMDA(1).GT.0.) .AND. AMDA(2).GT.0.) RETURN
    IF (AMDA(1).GT.0.) GO TO 60
    AMDA(1)=0.
    AMDA(2)=R(2)/D(2,2)
    ICON=1
    IF (AMDA(2).GT.0.) RETURN
    AMDA(2)=0.
    ICON=0
    RETURN
60  AMDA(1)=R(1)/D(1,1)
    AMDA(2)=0.
    ICON=1
    IF (AMDA(1).GT.0.) RETURN
    AMDA(1)=0.
    ICON=0
    RETURN
END

```

```

MAIN0380
MAIN0390
MAIN0400
MAIN0410
MAIN0420
MAIN0430
MAIN0440
MAIN0450
MAIN0460
MAIN0470
MAIN0480
MAIN0490
MAIN0500
MAIN0510
MAIN0520
MAIN0530
MAIN0540
MAIN0550
MAIN0560
MAIN0570

```

```

      SUBROUTINE MSGG(SF,DF,SFF,IW)
C*****
C-----PRINT SCALE FACTOR FOR SCALED DESIGN
C*****
      IF (SF.GF.DF) GO TO 550
      WRITE(IW,1003) SFF
      WRITE(IW,1004)
      GO TO 551
550  WRITE(IW,1003) SF
      WRITE(IW,1005)
551  RETURN
1003  FORMAT(//1X,15HSCALE FACTOR IS,F7.3,17HAND DETERMINED BY)
1004  FORMAT(1H+,40X,24HBUCKLING CONSTRAINTS )
1005  FORMAT(1H+,40X,18HSTRESS CONSTRAINTS )
      END

```

```

MAIN0580
MAIN0590
MAIN0600
MAIN0610
MAIN0620
MAIN0630
MAIN0640
MAIN0650
MAIN0660
MAIN0670
MAIN0680
MAIN0690
MAIN0700
MAIN0710
MAIN0720

```

```

SUBROUTINE TRUSS (A,M101)
C*****
C-----THREE DIMENSIONAL BAR ELEMENTS
C*****
DIMENSION A(M101)
COMMON /F1,PAR/ NPAR(14),MIMNP,MHANO,MFL1YP,N1,N2,N3,N4,N5,M11,NFO
1,MIMF1,MIMDV,M1,M2,M3,L1,LH,MFOH,NHLOCK
COMMON /HINK/ LT,LH,L,SIG(27),IDVAR,IFX,FRC,AREA,HIN1(334)
COMMON /UNIT/IR,IW,IP,I1,I2,I3,IR,I9,I10,I11,I12,I13
NIME=NPAP(2)
KODF=NPAP(5)
IF(NPAR(1).EQ.0)GO TO 500
GO TO (1,2),KODF
C*****
C-----KODF=1 INFERTIA IS PROPORTIONAL TO AREA FOR LOCAL BUCKLING
C-----KODF=2 INFERTIA IS PROPORTIONAL TO AREA**2 FOR LOCAL BUCKLING
C*****
1 MIMMAT=NPAP(3)
MIMGFO=NPAP(4)
MIMTC=NPAP(6)
N6=N5+MIMNP
N7=N6+MIMMAT
N8=N7+MIMMAT
N9=N8+MIMMAT+MIMTC*5
MM=N9+MIMGFO*2-M101
IF(MM.GT.0)CALL FPRPR(MM)
CALL TRUSS (A(M1),A(M1),A(N2),A(N2),A(N4),A(N5),A(N6),A(N7),
1A(N8),A(N9),MIMDV,MIMNP,MIMMAT,MIMTC,MIMGFO,KODF,NIME)
RETURN
C*****
C-----PROVISION FOR SPECIAL TRUSS ELEMENT
C*****
2 CALL NDFLEM (NPAK(1),NPAR(5),IW)
RETURN
500 WRITE (IW,2002) KODF
DO 800 MM=1,MIME
CALL STRSC(A(M1),A(N1),A(N3),NFO,MIMDV,L1,LH,0)
WRITE (IW,2005) MM,AREA
DO 800 I=L1,LH
CALL STRSC(A(M1),A(N1),A(N3),NFO,MIMDV,L1,LH,I)
IF(L.GT.1) WRITE (IW,2004)
WRITE (IW,2003) L,SIG(1)
GO TO (2,3,4),KODF
C*****
C-----DESIGN OF BAR ELEMENTS FOR STRESS AND LOCAL BUCKLING
C*****
3 CALL DTRUSS (A(M1),A(N2),A(N3),MIMDV)
GO TO 800
C*****
C-----PROVISION FOR DESIGN OF SPECIAL TRUSS ELEMENT
C*****
4 CONTINUE
800 CONTINUE
RETURN
2002 FORMAT(//42H ANALYSIS OF TRUSS ELEMENTS. CONSIDR CODE=,I2 //
1 47H ELEMENT X-SECT AREA LOAD COND AXIAL FORCE /)
2003 FORMAT(1H+,24Y,15,4X,F12,4)
2004 FORMAT(/)
2005 FORMAT(17,2Y,F12,4)
END

```

TRUSS0000  
TRUSS0010  
TRUSS0020  
TRUSS0030  
TRUSS0040  
TRUSS0050  
TRUSS0060  
TRUSS0070  
TRUSS0080  
TRUSS0090  
TRUSS0100  
TRUSS0110  
TRUSS0120  
TRUSS0130  
TRUSS0140  
TRUSS0150  
TRUSS0160  
TRUSS0170  
TRUSS0180  
TRUSS0190  
TRUSS0200  
TRUSS0210  
TRUSS0220  
TRUSS0230  
TRUSS0240  
TRUSS0250  
TRUSS0260  
TRUSS0270  
TRUSS0280  
TRUSS0290  
TRUSS0300  
TRUSS0310  
TRUSS0320  
TRUSS0330  
TRUSS0340  
TRUSS0350  
TRUSS0360  
TRUSS0370  
TRUSS0380  
TRUSS0390  
TRUSS0400  
TRUSS0410  
TRUSS0420  
TRUSS0430  
TRUSS0440  
TRUSS0450  
TRUSS0460  
TRUSS0470  
TRUSS0480  
TRUSS0490  
TRUSS0500  
TRUSS0510  
TRUSS0520  
TRUSS0530  
TRUSS0540  
TRUSS0550  
TRUSS0560  
TRUSS0570  
TRUSS0580  
TRUSS0590

```

      SUBROUTINE RUSS (IWT,X,Y,Z,T,NTC,WI,PMA1,PGFN,NUMDV,NUMNP,      TRUS0600
      1 NUMMAT,NUMTC,NUMGFN,KDOF,NIME)      TRUS0610
C-----UNIFORM CROSS SECTION BAR ELEMENTS      TRUS0620
C-----      TRUS0630
C-----      TRUS0640
      IMPLICIT REAL*8 (A-H,O-Z)      TRUS0650
      REAL*4 X,Y,Z,T,WI,PMA1,PGFN,IWT,FRG,HUCKYY,HUCKZZ,FF3,FF4      TRUS0660
      DIMENSION TH(NUMNP,6),X(NUMNP),Y(NUMNP),Z(NUMNP),T(NUMNP),      TRUS0670
      1NTC(NUMMAT),WT(NUMMAT),PMA1(NUMTC,5,NUMMAT),PGFN(NUMGFN,2),      TRUS0680
      2 IWT(NUMDV)      TRUS0690
      COMMON/EM/1,M(6),S(6,6),P(6,4),S1(6),1T(4),XM(6),G(6,6),FM1(2659)      TRUS0700
      COMMON/JUNK/FM1(4,4),FF(4),RHO,TEMP,XX(2),YY(2),ZZ(2),V(4),      TRUS0710
      1 JUNK(304)      TRUS0720
      COMMON/CONTR/IC(13),LHUCK,IC2(15)      TRUS0730
      COMMON/INTTS/IR,1W,1P,11,12,13,1R,19,110,111,112,113      TRUS0740
C-----CONTROL INFORMATION      TRUS0750
C-----CONTROL INFORMATION      TRUS0760
C-----      TRUS0770
      MI=1      TRUS0780
      MV=1      TRUS0790
      MW=1      TRUS0800
      MD=6      TRUS0810
      NS=1      TRUS0820
      NJ=4      TRUS0830
      NG=1      TRUS0840
      NSG=1      TRUS0850
      IFX=KDOF      TRUS0860
      WRITE(1W,2000)NIME,KDOF,NUMMAT,NUMTC,NUMGFN      TRUS0870
C-----      TRUS0880
C-----MATERIAL PROPERTY CARDS      TRUS0890
C-----      TRUS0900
      WRITE(1W,2001)      TRUS0910
      DO 10 J=1,NUMMAT      TRUS0920
      READ(1R,1001)N,NTC(N),WT(N)      TRUS0930
      IF (NTC(N).EQ.0) NTC(N)=1      TRUS0940
      WRITE(1W,2002)N,NTC(N),WT(N)      TRUS0950
C-----      TRUS0960
C-----TEMPERATURE DEPENDENT MATERIAL PROPERTIES      TRUS0970
C-----      TRUS0980
      NT=NTC(N)      TRUS0990
      DO 10 J=1,NT      TRUS1000
      READ(1R,2003) (PMA1(J,K,N),K=1,5)      TRUS1010
      IF (PMA1(J,5,N).EQ.0) PMA1(J,5,N)=PMA1(J,4,N)      TRUS1020
      IF (J.NE.1) WRITE(1W,2004)      TRUS1030
      10 WRITE(1W,2010) (PMA1(J,K,N),K=1,5)      TRUS1040
C-----      TRUS1050
C-----GEOMETRIC PROPERTY CARDS      TRUS1060
C-----      TRUS1070
      WRITE(1W,2006)      TRUS1080
      DO 51 J=1,NUMGFN      TRUS1090
      READ(1P,1006)N,AREA,(PGFN(N,J),J=1,2)      TRUS1100
      IF (AREA,LE.0.0) AREA=1.0      TRUS1110
      DO 80 J=1,2      TRUS1120
      80 IF (PGFN(N,J).EQ.0.) PGFN(N,J)=1000000.      TRUS1130
      WRITE(1W,2007)N,AREA,(PGFN(N,J),J=1,2)      TRUS1140
      AA=AREA**JFX      TRUS1150
      DO 51 J=1,2      TRUS1160
      51 PGFN(N,J)=9.4696*PGFN(N,J)/AA      TRUS1170
C-----      TRUS1180
C-----ELEMENT LOAD INPUT LERS      TRUS1190

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C*****TRUS1200
      READ(IR,1003)FMIH,TRUS1210
      WRITE(1W,2003)FMIH,TRUS1220
C*****TRUS1230
C-----FIFTH CARDS,TRUS1240
C*****TRUS1250
      WRITE(1W,2005)TRUS1260
      N=1,TRUS1270
      100 READ(IR,1004) IFL,I,I,J,I,IMAT,IGF0,INDV,FRC,REF1,FLPY,FLPZ,INC,TRUS1280
      IF(IFL,I,I,N) GO TO 700,TRUS1290
      IF(FPC,IF,0,0) FRC=1.0,TRUS1300
      IF(INC,F0,0) INC=1,TRUS1310
      IF(FLPY,IF,0,0) FLPY=1.0,TRUS1320
      IF(FLPZ,IF,0,0) FLPZ=1.0,TRUS1330
      KK=INC*(IFL-N),TRUS1340
      I=I-KK,TRUS1350
      J=J-KK,TRUS1360
      DO 500 NFI=N,IFL,TRUS1370
      XX(1)=X(I),TRUS1380
      XX(2)=X(J),TRUS1390
      YY(1)=Y(I),TRUS1400
      YY(2)=Y(J),TRUS1410
      Z(1)=Z(I),TRUS1420
      Z(2)=Z(J),TRUS1430
C*****TRUS1440
C-----INTERPOLATE MATERIAL PROPERTIES FOR AVERAGE ELEMENT TEMPERATURE,TRUS1450
C*****TRUS1460
      TEMP=0.5*(T(I)+T(J)),TRUS1470
      CALL INTERP(MAT,FF,NIMC,NIMMA,5,4,NIC(IMAT),IMAT,TEMP),TRUS1480
C*****TRUS1490
C-----FORM ELEMENT UNIFORM MATERIAL PROPERTIES,TRUS1500
C*****TRUS1510
      RHO=WT(IMAT),TRUS1520
      TEMP=TEMP-REF1,TRUS1530
      CALL TRUSS,TRUS1540
      IF(LBUCK,F0,0) GO TO 300,TRUS1550
      CALL TFORM(V(1),V(2),V(3),V(4)),TRUS1560
      CALL FLGSUM(G,NSG,ND,NG,I1),TRUS1570
      300 HH=FF(1)/(V(4)*V(4)),TRUS1580
      BUCKYY=PGFN(IGF0,1)*HH*FLPY,TRUS1590
      BUCKZ=PGFN(IGF0,2)*HH*FLPZ,TRUS1600
      FF3=FF(3),TRUS1610
      FF4=FF(4),TRUS1620
      HWI(INDV)=HWI(INDV)+RHO*(V(4)*FRC),TRUS1630
C*****TRUS1640
C-----FORM LOCATION MATRIX AND COMPUTE HAND WIDTH,TRUS1650
C*****TRUS1660
      DO 400 I=1,3,TRUS1670
      IM(I)=ID(I,I),TRUS1680
      400 IM(I+3)=ID(I,I),TRUS1690
      CALL CALRANIMJF,IM,5,P,ST,IT,NI,NV,NS,ND,NW,INDV,IFX,FRC),TRUS1700
      WRITE(1R) NI,BUCKYY,BUCKZ,FF3,FF4,TRUS1710
      WRITE(1W,2004) NFI,I,J,IMAT,IGF0,INDV,FRC,REF1,FLPY,FLPZ,NDIF,TRUS1720
C*****TRUS1740
C-----CHECK FOR MORE ELEMENTS,TRUS1740
C*****TRUS1750
      I=I+INC,TRUS1760
      J=J+INC,TRUS1770
      500 CONTINUE,TRUS1780
      N=IFL+1,TRUS1790

```



```

      IF(N,IF,TIME) GO TO 100
      RETURN
700 WRITE(14,2011)
      STOP
1001 FORMAT(2I5,F10.0)
1003 FORMAT(4F10.0)
1004 FORMAT(6I5,4F10.0,15)
1006 FORMAT(15,5X,3F10.0)
2000 FORMAT(44H)NUMBER OF TRUSS ELEMENTS      =,15/
      1      44H CONSTRUCTION CODE      =,15/
      2      44H NUMBER OF MATERIALS      =,15/
      3      44H NUMBER OF TEMPS FOR WHICH MATL PROPS GIVEN=,15/
      4      44H NUMBER OF DIFFERENT GEOMETRIES PROPS GIVEN=,15)
2001 FORMAT(// 25H MATERIAL PROPERTY CARDS //
      101H MATERIAL NUMBER SPECIFIC      YOUNGS      COEFFT
      2 OF /--ALLOWABLE STRESSES--// /
      301H NUMBER OF TEMPS WEIGHT      TEMP      MODULUS      THERM E
      4XPAN TENSION COMPRESSION /)
2002 FORMAT(16,5X,15,F12.4)
2003 FORMAT(// 25H ELEMENT LOAD MULTIPLIERS / 20X,1HA,14X,1HR,14X,1HC,
      1 14X,1HD,14H X-DIP,4F15.6/ 6H Y-DIR,4F15.6/ 6H Z-DIR,4F15.6/
      2 6H TEMP,4F15.6)
2004 FORMAT(17,1X,5I7,4F13.4,17)
2005 FORMAT(// 23H PROCESSED ELEMENT DATA//
      1116H ELEMENT /-MODE NOS-/ /--ELEMENT ID NOS-/ DESIGN VAR REFE
      2RENCE END FIXITY COEFFICIENTS      RAND /
      3116H NUMBER I      J      MATL GEOMY D VAR      FRACTION
      4EMP      YY      Z7      WIDTH /)
2006 FORMAT(// 25H GEOMETRIC PROPERTY CARDS//
      146H GEOMETRY X-SECT /--MOMENTS OF INERTIA--/ /
      246H NUMBER AREA      YY      ZZ /)
2007 FORMAT(16,2X,3F12.4)
2008 FORMAT(5F10.0)
2009 FORMAT(//)
2010 FORMAT(11H,30X,6F12.4)
2011 FORMAT(2RH TRUSS ELEMENT CARD IN ERROR )
      END

```

```

TRUS1800
TRUS1810
TRUS1820
TRUS1830
TRUS1840
TRUS1850
TRUS1860
TRUS1870
TRUS1880
TRUS1890
TRUS1900
TRUS1910
TRUS1920
TRUS1930
TRUS1940
TRUS1950
TRUS1960
TRUS1970
TRUS1980
TRUS1990
TRUS2000
TRUS2010
TRUS2020
TRUS2030
TRUS2040
TRUS2050
TRUS2060
TRUS2070
TRUS2080
TRUS2090
TRUS2100
TRUS2110
TRUS2120
TRUS2130
TRUS2140
TRUS2150
TRUS2160

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```

      SHORRTHINE TRUSS
C *****
C-----FORM TRUSS ELEMENT MATRICES
C *****
      IMPLICIT REAL*8 (A-H,O-Z)
      COMMON/PM/LM(6),S(6,6),P(6,4),SI(6),II(4),XM(6),G(6,6),FM(12659)
      COMMON/UNIK/FMU(14,4),F(4),RHO,TEMP,X(2),Y(2),Z(2),V(4),JUN(304)
      DIMENSION FMM(112)
      EQUIVALENCE (S,FMM)
      DO 5 I=1,112
      5 FMM(I)=0.
C *****
C-----COMPUTE UNIT STIFFNESS AND LOAD MATRICES
C *****
      CALL VECTOP (V,X(1),Y(1),Z(1),X(2),Y(2),Z(2))
      DO 10 I=1,3
      10 SI(I)=-V(I)/V(4)
      10 SI(I+3)=-SI(I)

```

```

TRUS2170
TRUS2180
TRUS2190
TRUS2200
TRUS2210
TRUS2220
TRUS2230
TRUS2240
TRUS2250
TRUS2260
TRUS2270
TRUS2280
TRUS2290
TRUS2300
TRUS2310
TRUS2320
TRUS2330
TRUS2340

```

```

      DO 300 I=1,6                                TRUS2350
      YY=ST(I)*F(I)*V(4)                            TRUS2360
      DO 250 K=1,6                                TRUS2370
      S(K,I)=ST(K)*YY                                TRUS2380
250   S(I,K)=S(K,I)                                TRUS2390
300   ST(I)=F(I)*S(I,I)                            TRUS2400
C*****TRUS2410
C-----GRAVITY AND THERMAL LOADS                TRUS2420
C*****TRUS2430
      FT=-FMP*F(1)*F(2)                            TRUS2440
      F=0.5*RH0*V(4)                                TRUS2450
      DO 350 I=1,4                                TRUS2460
      HH=FMIH(I,4)*F1                                TRUS2470
      TT(I)=HH                                        TRUS2480
      DO 250 M=1,3                                TRUS2490
      P(M,I)=FMIH(I,M)*F+HH*V(M)                    TRUS2500
350   P(M+3,I)=FMIH(I,M)*F-HH*V(M)                  TRUS2510
      RETURN                                          TRUS2520
      END                                            TRUS2530

```

```

      SUBROUTINE TGFDM(DX,DY,DZ,XI)                TRUS2540
C*****TRUS2550
C-----FORM TRUSS ELEMENT INITIAL GEOMETRIC STIFFNESS MATRIX IN GLOBAL TRUS2560
C      COORDINATES                                TRUS2570
C*****TRUS2580
      IMPLCT REFL*8 (A-H,N-7)                      TRUS2590
      COMMON/EM/LM(6),S(6,6),P(6,4),ST(6),TT(4),XM(6),G(6,6),FM(2659) TRUS2600
      G(1,1)=(1.0-DY*DZ)/XI                          TRUS2610
      G(1,2)=-DX*DY/XI                                TRUS2620
      G(1,3)=-DX*DZ/XI                                TRUS2630
      G(2,2)=(1.0-DY*DY)/XI                          TRUS2640
      G(2,3)=-DY*DZ/XI                                TRUS2650
      G(3,3)=(1.0-DZ*DZ)/XI                          TRUS2660
      G(2,1)=G(1,2)                                TRUS2670
      G(3,1)=G(1,3)                                TRUS2680
      G(3,2)=G(2,3)                                TRUS2690
      DO 100 J=1,3                                TRUS2700
      DO 100 I=1,3                                TRUS2710
      G(I+3,J+3)=G(I,J)                            TRUS2720
      G(I,J+3)=-G(I,J)                             TRUS2730
100   G(I+3,I)=-G(I,J)                             TRUS2740
      RETURN                                          TRUS2750
      END                                            TRUS2760

```

```

      SUBROUTINE DTRUSS( AOLD, ANEW, LPAO, NUMDV)          TRUS2770
C*****
C-----FULLY STRESSED DESIGN FOR TRUSS ELEMENTS          TRUS2780
C*****
      DIMENSION AOLD( NUMDV), ANEW( NUMDV), LPAO( NUMDV)    TRUS2790
      COMMON/ JUNK /      LT, LH, L, SG( 27), IDVAR, IFX, FRC, AFEA, XINERT, RYY,    TRUS2800
1      R77, TFNS, CDP1, JIN1( 329)
      P=SG( 1)
      IF( P.GT., 0.0) GO TO 100
      P1=CDP1*AFEA
      P2=0.5*P1
      P=-P
      PFY=XINERT*RYY
      XLY=P/PFY
      RMAX=SQRT( XLY)
      CALL JOHNS ( IFX, P, P1, P2, PFY, R)
      IF ( P.GT., RMAX) RMAX=R
      PF7=YINERT*R77
      XLY=P/PF7
      P=SQRT( XLY)
      IF ( R.GT., RMAX) RMAX=R
      CALL JOHNS ( IFX, P, P1, P2, PF7, R)
      IF ( R.GT., RMAX) RMAX=R
      GO TO 110
100  P1=TFNS*AFEA
      RMAX=P/P1
110  AA=RMAX*AOLD( IDVAR)
      IF( AA.LT., ANEW( IDVAR)) GO TO 60
      ANEW( IDVAR)=AA
      LPAO( IDVAR)=1.
60  CONTINUE
      RETURN
      END

```

TRUS2770  
 TRUS2780  
 TRUS2790  
 TRUS2800  
 TRUS2810  
 TRUS2820  
 TRUS2830  
 TRUS2840  
 TRUS2850  
 TRUS2860  
 TRUS2870  
 TRUS2880  
 TRUS2890  
 TRUS2900  
 TRUS2910  
 TRUS2920  
 TRUS2930  
 TRUS2940  
 TRUS2950  
 TRUS2960  
 TRUS2970  
 TRUS2980  
 TRUS2990  
 TRUS3000  
 TRUS3010  
 TRUS3020  
 TRUS3030  
 TRUS3040  
 TRUS3050  
 TRUS3060  
 TRUS3070  
 TRUS3080  
 TRUS3090

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      SUBROUTINE JOHNS ( I, A, R, C, D, AA)                TRUS3100
C*****
C-----JOHNSON'S PARABOLA USED FOR REDESIGN UNDER COMPRESSIVE FORCE TRUS3110
C*****
      GO TO ( 1, 2, 3), I
1      AA=R-( R-C)*C/D
      IF ( AA.LT., 0.00001) GO TO 50
      AA=A/AA
      RETURN
50  AA=0.0
      RETURN
2      AA=( A+( R-C)*C/D) /R
      RETURN
3      AA=SQRT( A**2+( 4*R*C*( R-C)/D) )
      AA=( AA+A) /2/R
      RETURN
      END

```

TRUS3100  
 TRUS3110  
 TRUS3120  
 TRUS3130  
 TRUS3140  
 TRUS3150  
 TRUS3160  
 TRUS3170  
 TRUS3180  
 TRUS3190  
 TRUS3200  
 TRUS3210  
 TRUS3220  
 TRUS3230  
 TRUS3240  
 TRUS3250  
 TRUS3260

```

      SUBROUTINE BEAM(A,MTO)
      *****
C-----THREE DIMENSIONAL BEAM ELEMENTS
C*****
      DIMENSION A(MTO)
      COMMON /ELPAR/ NPAR(14),NIMNP,MRAND,NELTYP,N1,N2,N3,N4,N5,MITT,NFOR
      1,NIMF1,NIMDV,M1,M2,M3,LL,LR,NFOR,NLOCK
      COMMON /JUNK/ I,T,L,H,L,SIG(27),IPVAR,IFX,FRG,ARFA,JUN1(334)
      COMMON /INITIS/ IR,IW,IP,I1,I2,I3,IR,I9,I10,I11,I12,I13
      NIMF=NPAR(2)
      KODF=NPAR(5)
      IF(NPAR(1),EQ,0) GO TO 500
      GO TO (1,2),KODF
C*****
C-----BEAM ELEMENTS WITH INSTABILITY CONSTRAINTS
C-----KODF =1 INSTABILITIES AND MODULI ARE PROPORTIONAL TO AREA
C      2 INSTABILITIES AND MODULI ARE PROPORTIONAL TO AREA**2 AND
C      ARFA**1.5 RESPECTIVELY
C*****
      1 NIMMAT=NPAR(3)
      NIMGF0=NPAR(4)
      NIMF1X=NPAR(6)
      IF (NIMF1X,EQ,0) NIMF1X=1
      NA=M5+NIMND
      N7=N6+NIMGF0
      NR=N7+NIMGF0*Q
      N9=NR+NIMMAT
      N10=N9+NIMMAT*5
      MM=N10+NIMF1X*2-MTO
      IF(MM,GT,0)CALL EXPROR(MM)
      CALL BEAM (A(M1),A(N1),A(N2),A(N3),A(N4),A(N6),A(N7),A(NR),A(N9),
      1A(N10),NIMDV,NIMNP,NIMGF0,NIMMAT,NIMF1X,KODF,NIMF,NPAR(6))
      RETURN
C*****
C-----PROVISION FOR SPECIAL BEAM ELEMENTS
C*****
      2 CALL NDFEM(NPAR(1),KODF,IW)
      RETURN
      500 WRITE (IW,2008) KODF
      DO 800 MM=1,NIMF
      CALL STRES(A(M1),A(N1),A(N2),NFO,NIMDV,LL,LR,0)
      WRITE (IW,2005) MM,ARFA
      DO 800 L=LT,LH
      CALL STRES(A(M1),A(N1),A(N3),NFO,NIMDV,LL,LR,1)
      IF(L,GT,1) WRITE(IW,2006)
      WRITE(A,2007) L,(SIG(I),I=1,12)
      GO TO (3,4),KODF
C*****
C-----DESIGN OF BEAM ELEMENTS FOR STRESS AND LOCAL BUCKLING CONSTRAINTS
C*****
      3 CALL DBEAM(A(M1),A(M2),A(M3),NIMDV)
      GO TO 800
C*****
C-----PROVISION FOR REDESIGN OF SPECIAL BEAM ELEMENTS
C*****
      4 CONTINUE
      800 CONTINUE
      RETURN
      2008 FORMATT(//4)H ANALYSIS OF BEAM ELEMENTS, CONSTRN CODE= ,I2//
      1104H ELEMENT X=SECT AREA LOAD CONN AXIAL RX SHEAR RY SHEAR

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2007 2AP R7 TORQUE MX MOMENT MY MOMENT MZ /) RFAM0600
2005 FORMAT(17,F13.4) RFAM0610
2006 FORMAT(/) RFAM0620
2007 FORMAT(1H+,20X,15.6X,6F12.4/32X,6F12.4) RFAM0630
END RFAM0640

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SURROUNTING RFAM0650
]NIMGEF0,NIMMAT,NIMFX,KODF,NIME,NIMFX] RFAM0660
C***** RFAM0670
C-----UNIFORM CROSS-SECTION BEAM ELEMENTS RFAM0680
C***** RFAM0690
IMPL[ CIT REFALSR (A-H,0-7) RFAM0700
PFA[ *4 IM1,X,Y,Z, PGF0, PMAT, SET, FRC RFAM0710
DIMENS[ DIM (IM1){NIMDV}, ID(NIMNP,6), X(NIMNP), Y(NIMNP), Z(NIMNP),
1 KSEC(NIMGEF0), PGF0 (NIMGEF0,9), PMAT (NIMMAT,6), SET (NIMFX,12) RFAM0730
2 ,MT (NIMMAT) RFAM0740
COMMON/EM/LH(24),S(24,24,2),P(24,4,2),ST(12,24,2),IT(12,4,2),
1 XM(24),C(24,24),FM(146) RFAM0750
COMMON/HINK/FMHU(3,4),T(3,3),IC(4),JC(12),XX(3),YY(3),ZZ(3),IF(3), RFAM0760
1 IX(3),IJ(3),DL, JIN(278) RFAM0780
COMMON/CONTP/IC(13),IBHCK,IC2(15) RFAM0790
COMMON/HINTS/IR,IS,IP,II,I2,I3,I8,I4,I10,I11,I12,I13 RFAM0800
C***** RFAM0810
C-----CONTROL INFORMATION RFAM0820
C***** RFAM0830
NI=2 RFAM0840
NV=2 RFAM0850
NS=12 RFAM0860
NW=2 RFAM0870
NI=10 RFAM0880
NC=1 RFAM0890
NSG=7 RFAM0900
IFX=KODF RFAM0910
WRITE (IW,2005) NIME,KODF,NIMMAT,NIMGEF0,NIMFX RFAM0920
C***** RFAM0930
C-----MATHEMATICAL PROPERTY CARDS RFAM0940
C***** RFAM0950
WRITE (IW,2001) RFAM0960
DO 10 J=1,NIMMAT RFAM0970
READ (IR,1001) N,MT(N),(PMAT(N,J),J=1,5) RFAM0980
IF (PMAT(N,4),IF,0.) PMAT(N,4)=PMAT(N,3) RFAM0990
IF (PMAT(N,5),IF,0.) PMAT(N,5)=0.577*PMAT(N,3) RFAM1000
WRITE (IW,2002) N,MT(N),(PMAT(N,J),J=1,5) RFAM1010
10 PMAT(N,2)=0.5*PMAT(N,1)/(1.+PMAT(N,2)) RFAM1020
C***** RFAM1030
C-----GEOMETRIC PROPERTY CARDS RFAM1040
C***** RFAM1050
WRITE (IW,2003) RFAM1060
DO 30 J=1,NIMGEF0 RFAM1070
READ (IR,1002) N,KSEC(N),AREA,( PGF0 (N,J),J=1,9) RFAM1080
IF (AREA,IF,0.) AREA=1.0 RFAM1090
IF (KSEC(N),FO,0) KSEC(N)=1 RFAM1100
IF (KSEC(N),NE,3) GO TO 15 RFAM1110
PGF0 (N,3)=PGF0 (N,2) RFAM1120
PGF0 (N,6)=0. RFAM1130
PGF0 (N,7)=0. RFAM1140

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      PGFN(N,8)=0.
      PGFN(N,9)=0.
15  WRITE(IW,2004) N,KSEC(N),ARFA,( PGFN(N,J),J=1,9)
      AA=ARFA*FX
      AAA=DSORT(ARFA*AA)
      DO 11 J=1,3
11  PGFN(N,J)=PGFN(N,J)/AA
      DO 12 J=4,9
12  PGFN(N,J)=PGFN(N,J)/AAA
30  CONTINUE
C*****
C-----ELEMENT INFORMATION
C*****
      READ(IR,1006) (IFMUL(I,J),J=1,4,I=1,3)
      WRITE(IW,2006) (IFMUL(I,J),J=1,4,I=1,3)
C*****
C-----FIXED-END FORCES
C*****
      IF(NIMEX.FO.O) GO TO 56
      WRITE(IW,2010)
      DO 55 I=1,NIMEX
      READ(IR,1005) N,(SFI(N,J),J=1,12)
55  WRITE(IW,2011) N,(SFI(N,J),J=1,12)
C*****
C-----ELEMENT CARDS
C*****
56  WRITE(IW,4000)
      N=1
100  READ(IR,3000) IFI,IF,IMAT,IGFC,IDV ,FRC,LC,JC,INC
      IF(FRC.FO.O) FRC=1.
      IF(INC.FO.O) INC=1
      KK=INC*(IFI-N)
      IX(1)=IF(1)-KK
      IX(2)=IF(2)-KK
      IX(3)=IF(3)-KK
      DO 500 NFI=N,IFI
      DO 120 I=1,3
      II=IX(I)
      XX(I)=X(II)
      YY(I)=Y(II)
120  ZZ(I)=Z(II)
C*****
C-----COMPUTE ELEMENT MATRICES
C*****
      RHQ=WT(I MAT)
      F =PMAT(I MAT,1)
      GC =PMAT(I MAT,2)
      AA=PGFN(IGFC,1)
      AA=PGFN(IGFC,2)
      AA7=PGFN(IGFC,3)
      CALL NEWRM(F,GC,RHQ,AA,AA,AA7,SFI,NIMEX,X,NFI,IW)
      IF(LBUCK.NF.O) CALL RGFNM
      WGT(IDV)=WGT(IDV)+01*RHQ*FRC
C*****
C-----FORM ELEMENT LOCATION MATRIX
C*****
      I=IX(1)
      J=IX(2)
      DO 170 M=1,6
      LM(M)=IN(I,M)

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```

RFAM1150
RFAM1160
RFAM1170
RFAM1180
RFAM1190
RFAM1200
RFAM1210
RFAM1220
RFAM1230
RFAM1240
RFAM1250
RFAM1260
RFAM1270
RFAM1280
RFAM1290
RFAM1300
RFAM1310
RFAM1320
RFAM1330
RFAM1340
RFAM1350
RFAM1360
RFAM1370
RFAM1380
RFAM1390
RFAM1400
RFAM1410
RFAM1420
RFAM1430
RFAM1440
RFAM1450
RFAM1460
RFAM1470
RFAM1480
RFAM1490
RFAM1500
RFAM1510
RFAM1520
RFAM1530
RFAM1540
RFAM1550
RFAM1560
RFAM1570
RFAM1580
RFAM1590
RFAM1600
RFAM1610
RFAM1620
RFAM1630
RFAM1640
RFAM1650
RFAM1660
RFAM1670
RFAM1680
RFAM1690
RFAM1700
RFAM1710
RFAM1720
RFAM1730
RFAM1740

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      IM(M+12)=0                                RFAM] 750
      IM(M+18)=0                                RFAM] 760
      170 IM(M+6)=J0(I,M)                       RFAM] 770
C*****RFAM] 780
C-----TRANSFORM TO MASTER DEGREES OF FREEDOM AND REARRANGE MATRICES RFAM] 790
C*****RFAM] 800
      MD=12
      CALL SLAVE (X,Y,Z,TD,MUMNP,IX(1),IX(2),ND,NS,IBUCK) RFAM] 820
      NM=ND*MD*MI RFAM] 830
      CALL REPARAN( S, S,24,24,2,ND,ND,MI,NM) RFAM] 840
      NM=NS*MD*MI RFAM] 850
      CALL REPARAN( ST,ST,12,24,2,NS,ND,MI,NM) RFAM] 860
      NM=ND*4*NV RFAM] 870
      CALL REPARAN( P, P,24, 4,2,ND, 4,NV,NM) RFAM] 880
C*****RFAM] 890
C-----PLACE ELEMENT INFORMATION ON TABLES RFAM] 900
C*****RFAM] 910
      CALL CALPAM(NDIF,IM,S,P,ST,TT,MI,NV,NS,ND,NW,INDV,IFX,FRG) RFAM] 920
      IF(IBUCK.EQ.0) GO TO 749 RFAM] 930
      NM=ND*MI*NG RFAM] 940
      CALL REPARAN(G,G,24,24,1,MI,ND,NG,NM) RFAM] 950
      CALL FLGSHW(G,MSG,ND,NG,111) RFAM] 960
      749 WRITE(18) MI,(PGFN(IGFN,I),I=4,9),(PMAT(1MA,I),I=3,5),KSFCL(IGFN) RFAM] 970
      WRITE(18,4001) NEL,IX,IMAT,IGFN,INDV,FRG,LC,JC,NDIF RFAM] 980
C*****RFAM] 990
C-----CHECK FOR LAST ELEMENT RFAM] 2000
C*****RFAM] 2010
      IX(1)=IX(1)+INC RFAM] 2020
      IX(2)=IX(2)+INC RFAM] 2030
      500 CONTINUE RFAM] 2040
      N=IFL+1 RFAM] 2050
      IF(N.LE.MIME) GO TO 100 RFAM] 2060
      RETURN RFAM] 2070
1001 FORMAT(15,5X,6F10.0) RFAM] 2080
1002 FORMAT(215,4F10.0/6F10.0) RFAM] 2090
1005 FORMAT(15,6F10.0/15,0,5F10.0) RFAM] 2100
1006 FORMAT(4F10.0) RFAM] 2110
2001 FORMAT(/ 25H MATERIAL PROPERTY CARDS // RFAM] 2120
      1P2H MATERIAL SPECIFIC YOUNGS POISSONS /-----ALLOWAR RFAM] 2130
      21F SIFSSFS-----/ / RFAM] 2140
      3P2H NUMBER WEIGHI MINIMUMS RATIO TENSION COMPRE RFAM] 2150
      4P2FSSION SHEAR /) RFAM] 2160
2002 FORMAT(16,4X,6F12.4) RFAM] 2170
2003 FORMAT(/ 25H GEOMETRIC PROPERTY CARDS // RFAM] 2180
      16AH PROPERTY X-SECT X-SECT /-----PROPERTIES OF X-SECTION--- RFAM] 2190
      2- / / RFAM] 2200
      36AH NUMBER KODE AREA X-AXIS Y-AXIS Z-AXIS RFAM] 2210
      4 /) RFAM] 2220
2004 FORMAT(16,4X,15,4F12.4,22H MOMENTS OF INERTIA / RFAM] 2230
      1 27X,3F12.4,24H SECT MODULI FOR POINT A/ RFAM] 2240
      2 27X,3F12.4,24H SECT MODULI FOR POINT B) RFAM] 2250
2005 FORMAT(32H)THREE DIMENSIONAL BEAM ELEMENTS// RFAM] 2260
      1 32H NUMBER OF BEAM ELEMENTS =.15/ RFAM] 2270
      2 32H CONSTRUCTION CODE =.15/ RFAM] 2280
      3 32H NUMBER OF MATERIALS =.15/ RFAM] 2290
      4 32H NUMBER OF GEOMETRIC PROPERTIES=.15/ RFAM] 2300
      5 32H NUMBER OF FIXED-END FORCE SETS=.15/ RFAM] 2310
2006 FORMAT(/ 25H ELEMENT LOAD MULTIPLIERS / 20X,1HA,14X,1HB,14X,1HC, RFAM] 2320
      1 14X,1HD,76H X-DIP,4F15.6/ 6H Y-DIP,4F15.6/ 6H Z-DIP,4F15.6/ ) RFAM] 2330
2010 FORMAT(141,40H FIXED END FORCES IN LOCAL COORDINATES RFAM] 2340

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1//50H TYPE      NODE      FORCE X      FORCE Y      FORCE Z      RFAM2350
2              35H MOMENT X      MOMENT Y      MOMENT Z      RFAM2360
2011 FORMAT(1H ,13.6X,1HT,3X,6F12.3/1H ,9X,1HJ,2X,6F12.3/) RFAM2370
3000 FORMAT (7I5,F10.0,4I5,12I1,I3) RFAM2380
4000 FORMAT(// 23H PROCESSED ELEMENT DATA// RFAM2390
1 106H ELEMENT /---NODE NDS---/ /---ELEMENT ID NDS---/ DESIGN VAR RFAM2400
2 107H FEM-FORCE ID FEM RELEASE CODES RAND / RFAM2410
3 107H NUMREP I JJ K MAIL GEDMY D VAR FRACTION RFAM2420
4 A B C D I J WIDTH /) RFAM2430
4001 FORMAT(17,2X,3I5,3I7,F12.4,2X,4I5,5X,6I1,5X,6I1,I6) RFAM2440
FEM RFAM2450

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SUBROUTINE CORR (A0,A1,A2,A3,Z) RFAM2460
C***** RFAM2470
C-----COMPUTES THE LARGEST REAL ROOT Z OF RFAM2480
C A0+A1*Z+A2*Z**2+A3*Z**3=0 RFAM2490
C***** RFAM2500
A0=A0/A3 RFAM2510
A1=A1/A3 RFAM2520
A2=A2/A3 RFAM2530
N=A1/3.0-A2**2/4.0 RFAM2540
R=(A1**A2-3.0*A0)/6.0-A2**3/27.0 RFAM2550
P=0**2+R**2 RFAM2560
IF (P.LT.0.0) GO TO 200 RFAM2570
P=SQRT(P) RFAM2580
RP=R+P RFAM2590
IF(RP.LT.0.) GO TO 50 RFAM2600
S1=RP**0.333333 RFAM2610
GO TO 60 RFAM2620
50 S1=(-RP)**0.333333 RFAM2630
60 RP=R-P RFAM2640
IF(RP.LT.0.) GO TO 70 RFAM2650
S2=RP**0.333333 RFAM2660
GO TO 80 RFAM2670
70 S2=(-RP)**0.333333 RFAM2680
80 Z=S1+S2-A2/3.0 RFAM2690
RETURN RFAM2700
200 P=-P RFAM2710
P=SQRT(P) RFAM2720
SBAR=SQRT(R**2+P**2) RFAM2730
COS3=R/SBAR RFAM2740
SIN3=P/SBAR RFAM2750
W=ATN(SIN3/COS3) RFAM2760
W=W/3.0 RFAM2770
C=COS(W) RFAM2780
S=SIN(W) RFAM2790
IF (S.LT.0.) S=-S RFAM2800
SBAR=SBAR**0.333333 RFAM2810
Z=2.0*SBAR*(C-A2/3.0 RFAM2820
R=-SBAR*(C-A2/3.0+1.73205I)*SBAR**S RFAM2830
IF (R.EQ.7) Z=R RFAM2840
RETURN RFAM2850
FEM RFAM2860

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SUBROUTINE NEWRM (F,GG,WT,AAZ,AAZ,AAZ,SF1,NHMFIX,NEL,IW)      RFAM2870
C*****RFAM2880
C-----CALCULATE ELEMENT MATRICES      RFAM2890
C*****RFAM2900
IMPLICIT REAL*8 (A-H,O-Z)      RFAM2910
REAL*8 SF1      RFAM2920
COMMON/EM/IN(24),S1(24,24),S2(24,24),P1(24,4),P2(24,4),S11(12,24),RFAM2930
1 ST2(12,24),T11(12,4),T12(12,4),XM(24),G(24,24),S(12,12),FM(12)      RFAM2940
COMMON/HUNK/FM11(3,4),T(3,3),LC(4),JC(12),XX(3),YY(3),ZZ(3),IF(3),RFAM2950
1 IX(3),II(4),V(4),W(4),R(12),JUM(238)      RFAM2960
DIMENSION SF1(NHMFIX,12),FMM(2040)      RFAM2970
F011VALFNCF (S1,FMM)      RFAM2980
DO 5 I=1,2040      RFAM2990
5 FMM(I)=0.      RFAM3000
DO 6 I=1,12      RFAM3010
DO 6 J=1,12      RFAM3020
6 S(I,J)=0.      RFAM3030
C*****RFAM3040
C-----FORM GLOBAL TO LOCAL COORDINATE TRANSFORMATION.      RFAM3050
C*****RFAM3060
CALL VECTOR(U,XX(1),YY(1),ZZ(1),XX(2),YY(2),ZZ(2))      RFAM3070
CALL VECTOR(V,XX(1),YY(1),ZZ(1),XX(3),YY(3),ZZ(3))      RFAM3080
HH=DOT(U,V)      RFAM3090
IF (ABS(HH)-1.0).LT.0.01 ) GO TO 40      RFAM3100
CALL CROSS(U,V,W)      RFAM3110
CALL CROSS(W,U,V)      RFAM3120
DO 30 I=1,3      RFAM3130
T(I,I)=U(I)      RFAM3140
T(2,I)=V(I)      RFAM3150
30 T(3,I)=W(I)      RFAM3160
C*****RFAM3170
C-----EXFND END FORCES IN LOCAL COORDINATES      RFAM3180
C*****RFAM3190
DO 73 N=1,4      RFAM3200
M=LC(N)      RFAM3210
IF (M.IF.0) GO TO 73      RFAM3220
DO 72 I=1,12      RFAM3230
72 T12(I,N)=SF1(M,I)      RFAM3240
73 CONTINUE      RFAM3250
C*****RFAM3260
C-----ELEMENT UNIT STIFFNESS MATRIX IN LOCAL COORDINATES S(I,J)      RFAM3270
C*****RFAM3280
DI=U(4)      RFAM3290
ZY=F/(DI*DI)      RFAM3300
COMMON7=ZY*AAZ      RFAM3310
COMMON7=ZY*AAZ      RFAM3320
S(1,1)=F/DI      RFAM3330
S(2,2)= COMMON7*12./DI      RFAM3340
S(3,3)= COMMON7*12./DI      RFAM3350
S(4,4)= GG*AAZ/DI      RFAM3360
S(5,5)= COMMON7* 4.*DI      RFAM3370
S(6,6)= COMMON7* 4.*DI      RFAM3380
S(2,6)= COMMON7* 6.      RFAM3390
S(3,5)= -COMMON7* 6.      RFAM3400
DO 102 I=1,6      RFAM3410
J=I+6      RFAM3420
102 S(I,J)=S(J,I)      RFAM3430
DO 104 I=1,4      RFAM3440
J=I+6      RFAM3450
104 S(I,J)=-S(J,I)      RFAM3460

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      S(5,11)= S(5,5)*0.5                      RFAM3470
      S(6,12)= S(6,6)*0.5                      RFAM3480
      S(2,12)= S(2,6)                          RFAM3490
      S(6, 8)=-S(2,6)                          RFAM3500
      S(8,12)=-S(2,6)                          RFAM3510
      S(3,11)= S(3,5)                          RFAM3520
      S(5, 9)=-S(3,5)                          RFAM3530
      S(9,11)=-S(3,5)                          RFAM3540
      DO 106 I=2,12                             RFAM3550
      K=I-1                                       RFAM3560
      DO 106 J=1,K                               RFAM3570
      106 S(I,J)=S(J,I)                          RFAM3580
C*****RFAM3590
C-----MODIFY S AND T12 FOR ZEPH-FORCES      RFAM3600
C*****RFAM3610
      DO 110 I=1,12                             RFAM3620
      DO 110 J=1,12                             RFAM3630
      110 G(I,J)=S(I,J)                          RFAM3640
      DO 140 I=1,12                             RFAM3650
      SJ=S(I,I)                                  RFAM3660
      IF(JC(1),LF,0,OR,SI,F0,0.) GO TO 140      RFAM3670
      DO 125 N=1,12                             RFAM3680
      125 R(N)=S(I,N)                            RFAM3690
      DO 126 N=1,4                              RFAM3700
      126 W(N)=T2(I,N)                           RFAM3710
      DO 135 M=1,12                             RFAM3720
      CM=S(M,I)/SI                               RFAM3730
      DO 130 N=1,12                             RFAM3740
      130 S(M,N)=S(M,N)-CM*W(N)                 RFAM3750
      DO 135 M=1,4                              RFAM3760
      135 T2(M,N)=T2(M,N)-CM*W(N)               RFAM3770
      140 CONTINUE                               RFAM3780
C*****RFAM3790
C-----INIT STIFFNESS AND FORCE RECOVERY MATRICES DUE TO STRETCHING RFAM3800
C*****RFAM3810
      DO 200 I=1,3                              RFAM3820
      DO 201 J=1,3                              RFAM3830
      X=T(1,I)*T(1,J)                           RFAM3840
      S1(I ,J )=X*S(1,1)                        RFAM3850
      S1(I ,J+6)=X*S(1,7)                      RFAM3860
      S1(I+6,J )=X*S(7,1)                      RFAM3870
      201 S1(I+6,J+6)=X*S(7,7)                  RFAM3880
      ST(1 ,J )=1(1,I)*S(1,1)                  RFAM3890
      X=T(1,I)*S(1,7)                           RFAM3900
      ST(1 ,I+6)=X                               RFAM3910
      ST(7 ,I )=X                               RFAM3920
      200 ST(7 ,I+6)=T(1,I)*S(7,7)             RFAM3930
      DO 202 I=1,7,6                             RFAM3940
      DO 202 J=1,7,6                             RFAM3950
      202 S(I,J)=0.                             RFAM3960
C*****RFAM3970
C-----INIT FORCE RECOVERY MATRIX DUE TO BENDING AND TWISTING RFAM3980
C*****RFAM3990
      DO 150 LA=1,10,3                          RFAM4000
      LR=LA+2                                    RFAM4010
      DO 150 MA=1,10,3                          RFAM4020
      MR=MA-1                                    RFAM4030
      DO 150 I=LA,LR                             RFAM4040
      DO 150 JM=1,3                              RFAM4050
      JM=IM+MR                                    RFAM4060

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      X=0.
      DO 151 K=1,3
      151 X=X+S(I,K+MR)*T(K,IM)
      150 S12(I,J)=X
C*****
C-----COORDINATE TRANSFORMATION OF UNIT BENDING AND TWISTING STIFFNESS
C*****
      DO 160 LA=1,10,3
      LA=LA-1
      DO 160 MA=1,10,3
      MR=MA+2
      DO 160 IL=1,3
      I=IL+1,R
      DO 160 J=MA,MR
      X=0.
      DO 161 K=1,3
      161 X=X+T(K,IL)*S12(K+1,R,J)
      160 S2(I,J)=X
C*****
C-----TRANSFORMATION OF ELEMENT LOAD VECTOR DUE TO FIXED END FORCES
C TO GLOBAL COORDINATES
C*****
      DO 165 LA=1,10,3
      LA=LA-1
      DO 165 IL=1,3
      I=IL+1,R
      DO 165 N=1,4
      X=0.
      DO 162 K=1,3
      162 X=X-T(K,IL)*T12(K+1,R,N)
      165 P2(I,N)=X
C*****
C-----ELEMENT MASS MATRIX
C*****
      X=WJ*DL/2.
      DO 180 M=1,3
      XM(M)=X
      XM(M+3)=0.
      XM(M+9)=0.
      180 XM(M+6)=X
C*****
C-----COMPUTE GRAVITY LOADING ( POINT LOADS ONLY )
C*****
      DO 190 J=1,3
      DO 190 I=1,4
      P1(I,J)=P1(I,J)+FMUL(I,J)*XM(I)
      190 P1(I+6,J)=P1(I+6,J)+FMUL(I,J)*XM(I+6)
      RETURN
      60 WRITE(10,4002) NEI
      STOP
4002 FORMAT (9HBEAM NO ,15, 26H K NOBE ON BEAM X-AXIS
. 26H.....EXCUTION TERMINATED )
      END

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BEAM4070
BEAM4080
BEAM4090
BEAM4100
BEAM4110
BEAM4120
BEAM4130
BEAM4140
BEAM4150
BEAM4160
BEAM4170
BEAM4180
BEAM4190
BEAM4200
BEAM4210
BEAM4220
BEAM4230
BEAM4240
BEAM4250
BEAM4260
BEAM4270
BEAM4280
BEAM4290
BEAM4300
BEAM4310
BEAM4320
BEAM4330
BEAM4340
BEAM4350
BEAM4360
BEAM4370
BEAM4380
BEAM4390
BEAM4400
BEAM4410
BEAM4420
BEAM4430
BEAM4440
BEAM4450
BEAM4460
BEAM4470
BEAM4480
BEAM4490
BEAM4500
BEAM4510
BEAM4520
BEAM4530
BEAM4540
BEAM4550
BEAM4560
BEAM4570
BEAM4580
BEAM4590

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SUBROUTINE SLAVE (X,Y,Z,IND,NUMNP,N1,N2,ND,NS,LRUCK)
C*****
C-----PERFORMS SLAVE ...MASTER DISPLACEMENT TRANSFORMATION
C      (FOR NODES CONNECTED TO BEAM ELEMENTS ONLY)
C*****
      IMPLICIT REAL*8 (A-H,O-Z)
      REAL*4 X,Y,Z
      COMMON/FM/IM(24),S(24,24,2),P(12),ST(12,24,2),TT(96),XM(24),
      I G(24,24),FM(146)
      DIMENSION X(NUMNP),Y(NUMNP),Z(NUMNP),IND(NUMNP,6)
C*****
C-----DETERMINE REQUIRED TRANSFORMATION DEGREES OF FREEDOM
C*****
      DO 54 NF=1,12,6
      ND=NI
      IF (NF,FO,7) ND=N.I
      DO 30 K=1,3
      I=K+NF-1
      IF (LM(I),GF,0) GO TO 30
      M=-LM(I)
      IM(I)=IND(M,K)
      N1=ND+1
      N2=ND+2
      IF(K-2) 35,45,55
25  N1=-(Y(ND)-Y(M))
      N2= 7*(ND)-7(M)
      LM(N1)=IND(M,6)
      LM(N2)=IND(M,5)
      GO TO 50
45  N1=-(7*(ND)-7(M))
      N2=  X(ND)-X(M)
      LM(N1)=IND(M,4)
      LM(N2)=IND(M,6)
      GO TO 50
55  N1=-(X(ND)-X(M))
      N2=  Y(ND)-Y(M)
      LM(N1)=IND(M,5)
      LM(N2)=IND(M,4)
      50 CONTINUE
C*****
C-----TRANSFORMATION INCREASE IM SIZE
C*****
      DO 60 J=1,2
      DO 60 I=1,ND
      S(N1,I,I)=S(I,I,I)*D1
      S(N2,I,I)=S(I,I,I)*D2
      S(I,I,M)=S(M,I,I)
60  S(I,I,N2,I)=S(N2,I,I,I)
      S(M,I,I)=S(I,I,I)*D1
      S(N1,N2,I)=S(I,I,I)*D1*D2
      S(N2,N1,I)=S(N1,N2,I,I)
      S(I,N2,N2,I)=S(I,I,I)*D2*D2
      DO 70 II=1,MS
      ST(I,I,N1,I)=ST(I,I,I)*D1
70  ST(I,I,N2,I)=ST(I,I,I)*D2
      80 CONTINUE
      XM(N1)=XM(I)*D1*D1
      XM(N2)=XM(I)*D1*D2
      IF(LRUCK,FO,0) GO TO 91
      DO 90 II=1,ND

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BEAM4600
BEAM4610
BEAM4620
BEAM4630
BEAM4640
BEAM4650
BEAM4660
BEAM4670
BEAM4680
BEAM4690
BEAM4700
BEAM4710
BEAM4720
BEAM4730
BEAM4740
BEAM4750
BEAM4760
BEAM4770
BEAM4780
BEAM4790
BEAM4800
BEAM4810
BEAM4820
BEAM4830
BEAM4840
BEAM4850
BEAM4860
BEAM4870
BEAM4880
BEAM4890
BEAM4900
BEAM4910
BEAM4920
BEAM4930
BEAM4940
BEAM4950
BEAM4960
BEAM4970
BEAM4980
BEAM4990
BEAM5000
BEAM5010
BEAM5020
BEAM5030
BEAM5040
BEAM5050
BEAM5060
BEAM5070
BEAM5080
BEAM5090
BEAM5100
BEAM5110
BEAM5120
BEAM5130
BEAM5140
BEAM5150
BEAM5160
BEAM5170
BEAM5180
BEAM5190

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      G(N1,I1)=G(I,I)*D1
      G(N2,I1)=G(I,I)*D2
      G(I1,N1)=G(N1,I1)
90   G(I1,N2)=G(N2,I1)
      G(N1,N1)=G(I,I)*D1*D1
      G(N1,N2)=G(I,I)*D1*D2
      G(N2,N1)=G(N1,N2)
      G(N2,N2)=G(I,I)*D2*D2
91   ND=ND+2
92   CONTINUE
C*****
C-----SET ROTATIONS
C*****
      ND=54, I=1,3
      K=NF+I+2
      IF(LM(K).GE.0) GO TO 54
      M=-LM(K)
      LM(K)=ID(M,I+3)
54   CONTINUE
      RETURN
      END

```

```

RFAM5200
RFAM5210
RFAM5220
RFAM5230
RFAM5240
RFAM5250
RFAM5260
RFAM5270
RFAM5280
RFAM5290
RFAM5300
RFAM5310
RFAM5320
RFAM5330
RFAM5340
RFAM5350
RFAM5360
RFAM5370
RFAM5380
RFAM5390
RFAM5400

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      SUBROUTINE RGFEM
C*****
C-----GLOBAL STIFFNESS MATRIX OF BEAM ELEMENT
C*****
      IMPLICIT REAL*8 (A-H,O-Z)
      COMMON/EM/LM(24),SS(1152),P(192),SI(576),TI(96),XM(24),G(24,24),
1   S(12,12),FMM(2)
      COMMON/JUNK/EMUL(3,4),T(3,3),LC(4),JC(12),X(3),Y(3),Z(3),IF(3),
1   IX(3),II(3),DI,C(3,3),R(12),JIN(236)
      ND=0, I=1,12
      DO 10 I=1,12
      ND=ND+1
      S(I,I)=0.
      D1=1./DI
      D2=0.13333333*D1
      D3=0.03333333*D1
      D4=0.1
      S(1,2)=D1
      S(3,3)=D1
      S(5,5)=D2
      S(6,6)=D2
      DO 11 J=2,6
11   S(I+6,I+6)=S(I,I)
      S(2,6)=D4
      S(2,8)=-D1
      S(2,12)=D4
      S(3,5)=-D4
      S(3,9)=-D1
      S(3,11)=-D4
      S(5,9)=D4
      S(5,11)=-D3
      S(6,8)=-D4
      S(6,12)=-D3
      S(8,12)=-D4
      S(9,11)=D4

```

```

RFAM5410
RFAM5420
RFAM5430
RFAM5440
RFAM5450
RFAM5460
RFAM5470
RFAM5480
RFAM5490
RFAM5500
RFAM5510
RFAM5520
RFAM5530
RFAM5540
RFAM5550
RFAM5560
RFAM5570
RFAM5580
RFAM5590
RFAM5600
RFAM5610
RFAM5620
RFAM5630
RFAM5640
RFAM5650
RFAM5660
RFAM5670
RFAM5680
RFAM5690
RFAM5700
RFAM5710
RFAM5720
RFAM5730
RFAM5740

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      DO 20 J=2,12                                RFAM5750
      I1=I-1                                        RFAM5760
      DO 20 J=1,I1                                  RFAM5770
      20 S(I,J)=S(I,J,I)                            RFAM5780
C*****RFAM5790
C-----MODIFY S(I,J) FOR ZERO END FORCES        RFAM5800
C*****RFAM5810
      DO 140 I=1,12                                RFAM5820
      IF(JC(I)).LE.0) GO TO 140                     RFAM5830
      ST=S(I,I)                                     RFAM5840
      IF(S(I,FO,0.)) GO TO 140                     RFAM5850
      IF(G(I,I),FO,0.)) GO TO 140                 RFAM5860
      DO 135 M=1,12                                RFAM5870
      135 R(M)=S(I,M)                               RFAM5880
      DO 130 M=1,12                                RFAM5890
      DO 130 M=1,12                                RFAM5900
      130 S(M,M)=S(M,M)-(G(M,I)*R(M)+G(M,J)*R(M))/G(I,I)
      1      +G(M,I)*G(M,I)*S/(G(I,I)*G(I,I))      RFAM5910
      DO 141 M=1,12                                RFAM5920
      S(I,M)=0.                                     RFAM5930
      141 S(M,I)=0.                                 RFAM5940
      140 CONTINUE                                  RFAM5950
C*****RFAM5960
C-----TRANSFORM TO GLOBAL COORDINATES          RFAM5970
C*****RFAM5980
      DO 150 I=1,24                                RFAM5990
      DO 150 J=1,24                                RFAM6000
      150 G(I,J)=0.                                  RFAM6010
      DO 250 J=1,4                                  RFAM6020
      I1=(I1-1)*3                                   RFAM6030
      DO 250 J1=1,11                                RFAM6040
      J1=(J1-1)*3                                   RFAM6050
      DO 260 J=1,3                                  RFAM6060
      DO 260 K=1,3                                  RFAM6070
      H=0.                                           RFAM6080
      DO 270 J1=1,3                                  RFAM6090
      270 H=H+S(I1+I,J1+J)*T(I,J,K)                RFAM6100
      260 G(I,K)=H                                   RFAM6110
      DO 280 J=1,3                                  RFAM6120
      DO 280 K=1,3                                  RFAM6130
      H=0.                                           RFAM6140
      DO 290 J1=1,3                                  RFAM6150
      290 H=H+T(I,J1)*C(J,K)                       RFAM6160
      280 G(I1+I,J1+K)=H                             RFAM6170
      250 CONTINUE                                  RFAM6180
      DO 300 I=1,12                                RFAM6190
      DO 300 J=1,I                                  RFAM6200
      300 G(I,I)=G(I,J)                             RFAM6210
      RETURN                                          RFAM6220
      END                                            RFAM6230

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      SUBROUTINE DREAM (ADLD,ANEW,LOAD,NUMDV)                                HFAM6250
C*****                                                                    HFAM6260
C-----DEFJGN OF BEAM ELEMENTS                                            HFAM6270
C*****                                                                    HFAM6280
      COMMON/JOINK/      LT,LH,L,SIG(27),IDVAR,IFX,FRC,ARFA,XINER1,          HFAM6290
      1 7FF(6),TENS,COMP,SHEAR,KSEC,SFCMOD(12),JIN1(311)                  HFAM6300
      DIMENSION ADLD(NUMDV),ANEW(NUMDV),LOAD(NUMDV)                        HFAM6310
      DFLT=0.001                                                            HFAM6320
      KMAX=6                                                                HFAM6330
      RMAX=0.                                                                HFAM6340
      AA=ARFA                                                                HFAM6350
      IF(IFX.F0.2) AA=SQRT(AA**3)                                          HFAM6360
      IF(KSEC.GT.1) GO TO 20                                                HFAM6370
C*****                                                                    HFAM6380
C-----SET UP SECTION MODULUS ARRAY SFCMOD(I) FOR ALL                     HFAM6390
C      FOUR STRESS POINTS OF X-SSECTION AT NODE I                         HFAM6400
C*****                                                                    HFAM6410
      DO 10 I=1,3                                                           HFAM6420
        SFCMOD(I)=7FF(I)*AA                                                HFAM6430
        SFCMOD(I+3)=SFCMOD(I)                                              HFAM6440
        SFCMOD(I+6)=-7FF(I+3)*AA                                          HFAM6450
      10 SFCMOD(I+9)=SFCMOD(I+6)                                          HFAM6460
        SFCMOD(I+2)=-SFCMOD(I)                                             HFAM6470
        SFCMOD(I+1)=-SFCMOD(I)                                             HFAM6480
      GO TO 25                                                              HFAM6490
C*****                                                                    HFAM6500
C-----SET UP SFCMOD(I) FOR 7-SECTION OR TUBE                             HFAM6510
C*****                                                                    HFAM6520
      DO 20 I=1,3                                                           HFAM6530
        SFCMOD(I)=7FF(I)*AA                                                HFAM6540
        SFCMOD(I+2)=-SFCMOD(I)                                             HFAM6550
        SFCMOD(I+6)=7FF(I+3)*AA                                          HFAM6560
      15 SFCMOD(I+9)=-SFCMOD(I+6)                                          HFAM6570
        SFCMOD(I+2)=-SFCMOD(I)                                             HFAM6580
        SFCMOD(I+5)=-SFCMOD(I)                                             HFAM6590
      IF(KSEC.NF.3) GO TO 25                                                HFAM6600
      SFCMOD(8)=0.                                                         HFAM6610
      SFCMOD(11)=0.                                                        HFAM6620
C*****                                                                    HFAM6630
C-----OBTAIN AXIAL FORCE X AND MOMENTS XX,YY,ZZ.                          HFAM6640
C      FIRST FOR NODE I, THEN FOR NODE J.                                  HFAM6650
C*****                                                                    HFAM6660
      25 X=SIG(7)                                                           HFAM6670
      DO 30 N=1,7,6                                                         HFAM6680
        IF(M.F0.1) GO TO 26                                                HFAM6690
      27 I=1.12                                                             HFAM6700
        SFCMOD(I)=-SFCMOD(I)                                               HFAM6710
      26 XX=SIG(N+3)                                                         HFAM6720
        YY=SIG(N+6)                                                         HFAM6730
        ZZ=SIG(N+9)                                                         HFAM6740
C*****                                                                    HFAM6750
C-----MOMENTY MOMENTS FOR TUBE                                           HFAM6760
C*****                                                                    HFAM6770
      IF(KSEC.NF.3) GO TO 40                                                HFAM6780
      YY=SQRT(YY*YY+ZZ*ZZ)                                                 HFAM6790
      ZZ=0.                                                                HFAM6800
      40 XAY[AI]=X/ARFA                                                    HFAM6810
C*****                                                                    HFAM6820
C-----COMPUTE STRESSES AT FOUR SECTION POINTS OF X-SSECTION             HFAM6830
C*****                                                                    HFAM6840

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DO 35 I=1,10,3                                HFAM6850
SSHEAR=0.                                        HFAM6860
IF (SECMOD(I),NF,0.) SSHEAR=XX/SECMOD(I)      HFAM6870
SRFND=0.                                        HFAM6880
IF (SECMOD(I+1),NF,0.) SRFND=YY/SECMOD(I+1)   HFAM6890
IF (SECMOD(I+2),NF,0.) SRFND=SRFND+77/SECMOD(I+2) HFAM6900
STOT=SAXIAL+SRFND                             HFAM6910
SSTAR=TFNS                                     HFAM6920
IF (STOT,LT,0.) SSTAR=-COMP                    HFAM6930
C*****HFAM6940
C-----APPLY REDESIGN FORMATIONS              HFAM6950
C*****HFAM6960
IF(IFX,FO,2) GO TO R1                          HFAM6970
R=SQRT((STOT/SSTAR)**2+(SSHEAR/SHEAR)**2)      HFAM6980
GO TO P2                                       HFAM6990
R1 IFEST=1                                     HFAM7000
TFST=((SSHEAR/SHEAR)**2-2.0*ABS(SAXIAL*SRFND))/SSTAR**2 HFAM7010
C*****HFAM7020
C-----CHECK IF SHEAR STRESS DOMINATES        HFAM7030
C*****HFAM7040
IF (TFST,GT,0.) IFEST=2                      HFAM7050
KQUINT=0                                     HFAM7060
RR=1.                                        HFAM7070
R=0.                                        HFAM7080
C=-SAXIAL/SSTAR                             HFAM7090
R5 GO TO (1,2),IFEST                         HFAM7100
1 AA=(SSHEAR/SHEAR/RR**3)**2                 HFAM7110
IF (AA,GT,1.) AA=0.                          HFAM7120
A=SQRT(1.-AA)                               HFAM7130
D=-SRFND/SSTAR                              HFAM7140
GO TO P0                                     HFAM7150
2 A=1.                                       HFAM7160
D=-*(SRFND/SSTAR)**2+(SSHEAR/SHEAR)**2+2.0*SAXIAL*SRFND/(SSTAR**2 HFAM7170
ISORT(RR))                                  HFAM7180
70 CALL CORR(D,C,R,A,R)                     HFAM7190
C*****HFAM7200
C-----CHECK FOR CONVERGENCE                  HFAM7210
C*****HFAM7220
IF (R,LT,0.000001) GO TO R0                 HFAM7230
DR=ABS((P-RR)/R)                             HFAM7240
IF (DR,LT,DEFI,TA,OR,KQUINT,FO,KMAX) GO TO R0 HFAM7250
KQUINT=KQUINT+1                             HFAM7260
RR=P                                         HFAM7270
GO TO R5                                     HFAM7280
R0 IF (IFEST,FO,1) R=R*R                     HFAM7290
R2 IF (R,GT,RMAX) RMAX=R                     HFAM7300
25 CONTINUE                                  HFAM7310
30 CONTINUE                                  HFAM7320
C*****HFAM7330
C-----REDORR NEW DESIGN VARIABLE AND CORRESPONDING LOAD CONDITION HFAM7340
C*****HFAM7350
AA=RMAX*ANLND(IDVAR)                         HFAM7360
IF(AA,LT,AMEW(IDVAR)) GO TO 60              HFAM7370
AMEW(IDVAR)=AA                               HFAM7380
LOAD(IDVAR)=I                               HFAM7390
60 CONTINUE                                  HFAM7400
RETURN                                       HFAM7410
END                                         HFAM7420

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SURROUNDING PLANE( A,MTOT)                                PL AN0000
C *****PI AN0010
C-----PLANE STRESS ELEMENTS                                PL AN0020
C *****PI AN0030
      DIMENS(10) A(MTOT), STRLAR(5)                        PL AN0040
      COMMON /FLPAR/ NPAR(14),NIMNP,MHANO,NFLTP,N1,N2,N3,N4,N5,M111,NFOPI AN0050
      NIMFI,NIMDV,M1,M2,M3,LL,LR,NFOR,NHLOC                                PL AN0060
      COMMON/EM/M1,M2,M3,N1,N2,N3,N4,N5,N6,N7,N8,N9,N10,N11,N12,N13
      COMMON/JUNK/ LL,LR,LL,SG(20),SIG(7),IDV,IFX,FRC,ARFA,XINERT,PL AN0080
      1 DEFINE(333)                                PL AN0090
      COMMON/UNITS/IR,IR,IP,I1,I2,I3,IR,I9,I10,I11,I12,I13                                PL AN0100
      DATA STRLAR/3HCFN,3H1-I,3H1-K,3H1-L,3HK-L/                                PL AN0110
      NIME=NPAR(2)                                PL AN0120
      KODE=NPAR(5)                                PL AN0130
      IF(NPAR(1),FO,0)GO TO 500                                PL AN0140
      N6=N5+NIMNP                                PL AN0150
      NIMMA1=NPAR(3)                                PL AN0160
      NIM7C=NPAR(4)                                PL AN0170
      GO TO (1,2,3),KODE                                PL AN0180
C *****PI AN0190
C-----UNIT REFERENCE STIFFNESS PANEL                                PL AN0200
C *****PI AN0210
      1 NIMGE1=NPAR(7)                                PL AN0220
      N7=N6+NIMMA1                                PL AN0230
      NR=N7+NIMMA1                                PL AN0240
      NQ=NR+NIMGE1*5                                PL AN0250
      N1=NQ+NIMMA1*NIM7C*8                                PL AN0260
      MM=N1*O-MTOT                                PL AN0270
      IF(MM,GT,0) CALL FPROR(MM)                                PL AN0280
      CALL PLMAX1(A(M1),A(N1),A(N2),A(N3),A(N4),A(N5),A(N6),A(N7),A(N8),PI AN0290
      A(N9),NIMDV,NIMNP,NIMMA1,NIM7C,KODE,NIME,NIMGE1)                                PL AN0300
      RETURN                                PL AN0310
C *****PI AN0320
C-----ISOTROPIC PLANE MEMBRANE                                PL AN0330
C *****PI AN0340
      2 N7=N6+NIMMA1                                PL AN0350
      NR=N7+NIMMA1                                PL AN0360
      NQ=NR+NIMMA1*NIM7C*7                                PL AN0370
      MM=NQ-MTOT                                PL AN0380
      IF(MM,GT,0) CALL FPROR(MM)                                PL AN0390
      CALL PLMAX2(A(M1),A(N1),A(N2),A(N3),A(N4),A(N5),A(N6),A(N7),A(N8),PI AN0400
      NIMDV,NIMNP,NIMMA1,NIM7C,KODE,NIME)                                PL AN0410
      RETURN                                PL AN0420
C *****PI AN0430
C-----PROVISION FOR SPECIAL MEMBRANE ELEMENT                                PL AN0440
C *****PI AN0450
      3 CALL NDEFEM (NPAR(1),KODE,14)                                PL AN0460
      RETURN                                PL AN0470
500 WRITE (14,2008) KODE                                PL AN0480
      DO 800 MM=1,NIME                                PL AN0490
      CALL STRSC(A(M1),A(N1),A(N3),NFO,NIMDV,LL,LR,0)                                PL AN0500
      WRITE (14,2005) MM,ARFA                                PL AN0510

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      IF(NS,FO,1) GO TO 800
      DO 700 I=1,T,1H
      CALL STPSC(A(M1),A(M1),A(M3),NFO,NUMMOV,I,L,LB,1)
      IF(L,GT,1) WRITF(1W,2007)
      IT=0
      DO 600 KK=1,NS,3
      IF(KK,GT,1) WRITF(1W,2007)
      DO 520 I=1,3
520  SIG(I)=SG(KK-1+I)
      IT=IT+1
      NP=3
      IF(IT,GT,1) GO TO 530
      NP=6
      AMG=DEFIME(1)
      IF(AMG,NF,0.) GO TO 540
      DO 550 J=1,3
550  SIG(1+3)=SIG(1)
      GO TO 530
540  AMG=AMG/57.2957795
      SI=SIN(AMG)
      CO=COS(AMG)
      SC=SI*CO
      ST=SI*SI
      CN=CO*CO
      X1=SIG(1)*CN+SIG(2)*SI
      X2=2.0*SIG(3)*SC
      SIG(4)=X1+X2
      SIG(5)=X1-X2
      SIG(6)=(SIG(2)-SIG(1))*SC+SIG(3)*(CN-SI)
530  GO TO (4,5,6),KIDF
C*****
C-----DESIGN OF STIFFENED MEMBRANE ELEMENT
C*****
4  IF(IT,GT,1) GO TO 600
      CALL DPLAN1(A(M1),A(M2),A(M3),NUMMOV)
      GO TO 600
C*****
C-----DESIGN OF ISOTROPIC MEMBRANE ELEMENT
C*****
5  CALL DPLAN2(A(M1),A(M2),A(M3),NUMMOV)
      GO TO 600
C*****
C-----PROVISION FOR DESIGN OF SPECIAL MEMBRANE ELEMENT
C*****
6  CONTINUE
      600 WRITF(1W,2009) L,STRLEN(1),SIG(1),I=1,NP)
      700 CONTINUE
      800 CONTINUE
C*****
      RETURN
      2005 FORMAT(1X,14,F14.4)
      2008 FORMAT(//45H ANALYSIS OF MEMBRANE ELEMENTS. CONSTRN CODE=,I2//
      1117H SHEET L1AD /---MEMBRANE FORCES IN
      210CAL COORDS---//---MEMBRANE FORCES IN MATERIAL COORDS-/ /
      3117H ELEMENT THICKNESS CONN LOCATION NXX NYY
      4 NXY N11 N22 N12 /)
      2009 FORMAT(11H+,20X,15.6X,A3,4X,6F13.4)
      2007 FORMAT(/)
      END

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PI AN0520
PI AN0530
PI AN0540
PI AN0550
PI AN0560
PI AN0570
PI AN0580
PI AN0590
PI AN0600
PI AN0610
PI AN0620
PI AN0630
PI AN0640
PI AN0650
PI AN0660
PI AN0670
PI AN0680
PI AN0690
PI AN0700
PI AN0710
PI AN0720
PI AN0730
PI AN0740
PI AN0750
PI AN0760
PI AN0770
PI AN0780
PI AN0790
PI AN0800
PI AN0810
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PI AN0900
PI AN0910
PI AN0920
PI AN0930
PI AN0940
PI AN0950
PI AN0960
PI AN0970
PI AN0980
PI AN0990
PI AN1000
PI AN1010
PI AN1020
PI AN1030
PI AN1040
PI AN1050
PI AN1060
PI AN1070
PI AN1080
PI AN1090
PI AN1100

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      SUBROUTINE FEA1(REF1A)                                PL AN1110
C*****                                                    PL AN1120
C-----STRESS /STRAIN RELATION MATRIX                    PL AN1130
C*****                                                    PL AN1140
      IMPLCTT REA1=PI*(A-H,N-7)                               PL AN1150
      COMMON/UNIK/IF(4),IX(4),FMUL(4,5),N(3,3),XX(4),YY(4),ZZ(4),TMP(4),PL AN1160
      IALP(3),TTT(3),PRESS,PFFT,NS,MSG(3),T(3,3),DD(3,3),JIN1(2)4) PL AN1170
      IF(REF1A.EQ.0.0) GO TO 500                               PL AN1180
      ANG=REF1A/57.2957795                                       PL AN1190
      SS=DSIN(ANG)                                                PL AN1200
      CO=DCOS(ANG)                                                PL AN1210
      C2=C0*CO                                                    PL AN1220
      S2=SS*SS                                                    PL AN1230
      SC=SS*CO                                                    PL AN1240
C*****                                                    PL AN1250
C-----SET D FOR SIG(N)=D*SIG(G)                          PL AN1260
C*****                                                    PL AN1270
      T(1,1)=C2                                                  PL AN1280
      T(1,2)=S2                                                  PL AN1290
      T(1,3)=2.*SC                                              PL AN1300
      T(2,1)=S2                                                  PL AN1310
      T(2,2)=C2                                                  PL AN1320
      T(2,3)=-2.*SC                                             PL AN1330
      T(3,1)=-SC                                                PL AN1340
      T(3,2)=SC                                                 PL AN1350
      T(3,3)=C2-S2                                              PL AN1360
      DD(3,0)=1,3                                              PL AN1370
      DD(3,0)=1,3                                              PL AN1380
      SUM=0.                                                    PL AN1390
      DD(2,0)=1,3                                              PL AN1400
280  SUM=SUM+T(M,I)*D(M,I)                                     PL AN1410
300  DD(I,I)=SUM                                               PL AN1420
      DD(3,0)=1,3                                              PL AN1430
      DD(3,0)=1,3                                              PL AN1440
      SUM=0.                                                    PL AN1450
      DD(3,0)=1,3                                              PL AN1460
320  SUM=SUM+DD(I,M)*T(M,I)                                   PL AN1470
      DD(I,I)=SUM                                              PL AN1480
350  D(I,I)=SUM                                               PL AN1490
      A1=A1,P(1)                                               PL AN1500
      A2=A1,P(2)                                               PL AN1510
      A1,P(1)=C2*A1+S2*A2                                       PL AN1520
      A1,P(2)=S2*A1+C2*A2                                       PL AN1530
      A1,P(3)=2.*SC*SIG(A1-A2)                                  PL AN1540
500  CALL PDSINV(D)                                           PL AN1550
      DD(6,0)=1,3                                              PL AN1560
      TTT(I)=0.                                                PL AN1570
      DD(6,0)=1,3                                              PL AN1580
670  TTT(I)=TTT(I)+D(I,M)*A1,P(M)                             PL AN1590
      PFTURN                                                    PL AN1600
      FEND                                                    PL AN1610

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      SHORROUTIME QHIAN(RHP,THICK)                                PL AN1620
C*****                                                                    PL AN1630
C-----FORM ELEMENT MATRIX CFS                                    PL AN1640
C*****                                                                    PL AN1650
      1MPLTCT REAL*8 (A-H,0-7)                                PL AN1660
      COMMON/FM/LN(12),S(12,12),P(12,4),P2(12,4),XM(12),S1(15,12),    PL AN1670
      1 T(15,4),G1(12,12),G2(12,12),G3(12,12),RH(12,12),FM(1700)    PL AN1680
      COMMON/UNK/UF(4),UX(4),FMUL(4,5),D(3,3),X(4),Y(4),Z(4),TH(4),    PL AN1690
      1 ALP(3),TTI(3),PRESS,REFI,ANS,NSG(3),WR(4),ZZ(4),PPI(12),    PL AN1700
      2 U(4),V(4),W(4),H(4),HP(4),H2(4),FAC,G(4),F(4),JIN(132)    PL AN1710
      COMMON/ELPAR/NPAR(14),TFLP(14)                                PL AN1720
      COMMON/CONTR/IC1(13),LHICK,IC2(15)                            PL AN1730
      DIMENS ION SS(2),SSS(5),IT(5),IVFCT(4),JVFCT(4),FMM(1068)    PL AN1740
      EQUIVALENCE (S,FMM)                                            PL AN1750
      DATA SS/-0.57735026918963,0.57735026918963/                PL AN1760
      DATA SSS/0.,-1.,0.,0.,0./, TTI/0.,0.,0.,-1.,1./            PL AN1770
      DATA IVFCT/4,2,1,3/,JVFCT/1,3,2,4/                          PL AN1780
      DO 10 I=1,1068                                                PL AN1790
      10 FMM(I)=0.                                                    PL AN1800
      DO 20 I=1,12                                                  PL AN1810
      20 PPI(I)=0.                                                    PL AN1820
C*****                                                                    PL AN1830
C-----COMPUTE ELEMENT AXES SYSTEM AND CORNER COORDINATES        PL AN1840
C*****                                                                    PL AN1850
      CALL VECTOP(U,X(1),Y(1),Z(1),X(2),Y(2),Z(2))                PL AN1860
      CALL VECTOP(F,X(1),Y(1),Z(1),X(4),Y(4),Z(4))                PL AN1870
      CALL CROSS(U,F,W)                                              PL AN1880
      CALL CROSS(W,U,V)                                              PL AN1890
      CALL VECTOP(G,X(1),Y(1),Z(1),X(3),Y(3),Z(3))                PL AN1900
      RR(1)=0.0                                                      PL AN1910
      ZZ(1)=0.0                                                      PL AN1920
      RP(2)=U(4)                                                     PL AN1930
      ZZ(2)=0.0                                                      PL AN1940
      RP(3)=G(4)*DOT(G,U)                                            PL AN1950
      ZZ(3)=G(4)*DOT(G,V)                                            PL AN1960
      RP(4)=F(4)*DOT(F,U)                                            PL AN1970
      ZZ(4)=F(4)*DOT(F,V)                                            PL AN1980
C*****                                                                    PL AN1990
C-----FORM UNIT STIFFNESS MATRIX , THERMAL LOAD VECTOR AND MASS MATRIX PL AN2000
C*****                                                                    PL AN2010
      DO 500 I=1,2                                                  PL AN2020
      DO 500 J=1,2                                                  PL AN2030
      CALL FORMR(SS(I,J),SSS(J,I),RH)                                PL AN2040
      F10=H(I)*TM(1)+H(2)*TM(2)+H(3)*TM(3)+H(4)*TM(4)-REFI        PL AN2050
      DO 400 J=1,12                                                  PL AN2060
      R1=RR(J,1)*FAC                                                PL AN2070
      R2=RR(J,2)*FAC                                                PL AN2080
      R3=RR(J,3)*FAC                                                PL AN2090
      D1=D(1,1)*R1+D(1,2)*R2+D(1,3)*R3                              PL AN2100
      D2=D(2,1)*R1+D(2,2)*R2+D(2,3)*R3                              PL AN2110
      D3=D(3,1)*R1+D(3,2)*R2+D(3,3)*R3                              PL AN2120
      PPI(J)=PPI(J)+F1P*(D1*ALP(1)+D2*ALP(2)+D3*ALP(3))            PL AN2130
      DO 400 I=1,12                                                  PL AN2140
      S(I,J)=S(I,J)+RR(I,J)*D1+RR(2,J)*D2+RR(3,J)*D3              PL AN2150
      400 S(J,I)=S(I,J)                                              PL AN2160
      DO 450 I=1,4                                                  PL AN2170
      450 XM(I)=XM(I)+FAC*(HICK*U(I))                                PL AN2180
      IF(LHICK,END) GO TO 500                                         PL AN2190
C*****                                                                    PL AN2200
C-----FORM UNIT GEOMETRIC STIFFNESS MATRIX CFS                    PL AN2210

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C*****PI AN2220
      DO 850 I=1,4                                PL AN2230
      DO 850 J=1,4                                PL AN2240
      I4=I+4                                       PL AN2250
      J4=J+4                                       PL AN2260
      IR=I+8                                       PL AN2270
      JR=J+8                                       PL AN2280
      XX=HR(I)*HR(J)*FAC                         PL AN2290
      YY=HZ(I)*HZ(J)*FAC                         PL AN2300
      XY=HR(I)*HZ(J)*FAC                         PL AN2310
      YX=HZ(I)*HR(J)*FAC                         PL AN2320
      G1(I,J)=G1(I,J)+YY                          PL AN2330
      G1(I,J4)=G1(I,J4)-YX                       PL AN2340
      G1(J4,J)=G1(I,J4)                          PL AN2350
      G1(J4,J4)=G1(J4,J4)+XX                     PL AN2360
      G1(IR,JR)=G1(IR,JR)+XY                     PL AN2370
      G2(IR,JR)=G2(IR,JR)+YY                     PL AN2380
850  G3(IR,JR)=G3(IR,JR)+XY+YX                  PL AN2390
500  CONTINUE                                     PL AN2400
      IF(LRICK.FO.O) GO TO 900                   PL AN2410
      DO 950 I=1,8                                PL AN2420
      DO 950 J=1,8                                PL AN2430
      G1(I,J)=G1(I,J)*.25                        PL AN2440
950  G2(I,J)=G1(I,J)                             PL AN2450
      CALL PLANCET (G1,RR,II,V,W)                PL AN2460
      CALL PLANCET (G2,RR,II,V,W)                PL AN2470
      CALL PLANCET (G3,RR,II,V,W)                PL AN2480
C*****PI AN2490
C-----FORM STRESS DISPLACEMENT MATRIX        PL AN2500
C*****PI AN2510
900  IL=NS/3                                       PL AN2520
      DO 530 I=1,IL                               PL AN2530
      CALL FORMR(SSS(I),TTT(I),RR)               PL AN2540
      FIP=H(I)*TM(1)+H(2)*TM(2)+H(3)*TM(3)+H(4)*TM(4)-RFEI PL AN2550
      DO 530 J=1,3                                PL AN2560
      I=I+3*(J-1)                                PL AN2570
      TT(I,4)=-TTT(I)*FIP                        PL AN2580
      DO 530 J=1,12                              PL AN2590
      DO 530 K=1,3                                PL AN2600
830  ST(I,J)=ST(I,J)+D(I,J,K)*RR(K,J)          PL AN2610
C*****PI AN2620
C-----FINITE EXTRA DEGREES OF FREEDOM        PL AN2630
C*****PI AN2640
      IF (IX(3).FO. IX(4)) GO TO 560             PL AN2650
      IF(NPAR(6).NE.O) GO TO 560                 PL AN2660
      DO 550 NM=1,4                               PL AN2670
      L=J2-NM                                     PL AN2680
      K=L+1                                       PL AN2690
      C=PP1(K)/S(K,K)                             PL AN2700
      DO 535 J=1,NS                               PL AN2710
535  TT(J,4)=TT(J,4)+C*ST(I,J,K)               PL AN2720
      DO 550 I=1,IL                               PL AN2730
      C=S(I,K)/S(K,K)                             PL AN2740
      PP1(I)=PP1(I)-C*PP1(K)                     PL AN2750
      DO 540 J=1,NS                               PL AN2760
540  ST(J,I)=ST(J,I)-C*ST(I,J,K)               PL AN2770
      DO 550 J=1,IL                               PL AN2780
550  ST(I,J)=S(I,J)-C*S(K,J)                   PL AN2790
C*****PI AN2800
C-----FINITE STRESS-DISPLACEMENT TRANSFORMATION TO GIVE STRESSES PL AN2810

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C      NORMAL AND PARALLEL TO SIDES - SIMILARLY ROTATE INITIAL STRESSES PL AN2820
C *****
560 MSET=L-1
      IF ( MSET .LE. 0 ) GO TO 730
      DO 720 I=1,MSET
          IV=VFC(I,I)
          JV=VFC(I,J)
          CM1=VFC(JNR(G,RR(IV),77(IV),0.000,RR(JV),77(JV),0.000))
          S2=G(1)*G(1)
          C2=G(2)*G(2)
          SC=-G(1)*G(2)
          T1=2*I+1
          T2=T1+1
          T3=T1+2
          T4=T1(I+.4)
          T2=T1(I2+.4)
          T3=T1(I3+.4)
          T4=2.0*SC*T3
          TT(I1,4)=C2*T1+S2*T2+T4
          TT(I2,4)=S2*T1+C2*T2-T4
          TT(I3,4)=SC*(T2-T1)+(C2-S2)*T3
          DO 710 J=1,8
              R1=ST(I1,J)
              R2=ST(I2,J)
              R3=ST(I3,J)
              R4=2.0*SC*R3
              ST(I1,J)=C2*R1+S2*R2+R4
              ST(I2,J)=S2*R1+C2*R2-R4
          710 ST(I3,J)=SC*(R2-R1)+(C2-S2)*R3
          720 CONTINUE
          730 IF(NPAR(5),NF,2) GO TO 150
C *****
C-----CALCULATE PRESSURE LOADS ON I-J FACE IN GLOBAL COORDINATES
C *****
      XX=0.5*PRFCS*RR(2)
      DO 185 I=1,3
          T1=(I-1)*4+1
          DO 185 L=1,4
              P2(I,L)=XX*V(I)*FMIL(I,2)
          185 P2(I+1,L)=P2(I,L)
C *****
C-----COORDINATE TRANSFORMATION
C *****
      150 DO 190 J=1,3
          DO 190 K=1,4
              KK=4*(J-1)+K
              DO 180 I=1,3
                  DO 180 L=1,4
                      IL=4*(I-1)+L
                      RR(KK,IL)=H(I)*S(K,L)*H(I)+S(K,L+4)*V(I)+
                      1 V(I)*S(K+4,L)*H(I)+S(K+4,L+4)*V(I)
                      X1=H(I)*PP(I,K)+V(I)*PP(I,K+4)
                  190 P1(KK,IL)=X1*FMIL(I,J)
              DO 195 I=1,12
                  DO 195 J=I,12
                      S(I,J)=RR(I,J)
          195 S(J,I)=S(I,J)
          DO 200 K=1,N5
              DO 200 I=1,4

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PL AN2840
PL AN2850
PL AN2860
PL AN2870
PL AN2880
PL AN2890
PL AN2900
PL AN2910
PL AN2920
PL AN2930
PL AN2940
PL AN2950
PL AN2960
PL AN2970
PL AN2980
PL AN2990
PL AN3000
PL AN3010
PL AN3020
PL AN3030
PL AN3040
PL AN3050
PL AN3060
PL AN3070
PL AN3080
PL AN3090
PL AN3100
PL AN3110
PL AN3120
PL AN3130
PL AN3140
PL AN3150
PL AN3160
PL AN3170
PL AN3180
PL AN3190
PL AN3200
PL AN3210
PL AN3220
PL AN3230
PL AN3240
PL AN3250
PL AN3260
PL AN3270
PL AN3280
PL AN3290
PL AN3300
PL AN3310
PL AN3320
PL AN3330
PL AN3340
PL AN3350
PL AN3360
PL AN3370
PL AN3380
PL AN3390
PL AN3400
PL AN3410

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```

      DO 200 J=1,3
      L1=4*(J-1)+1
200  PP1(L1)=ST(K,L)*II(J)+ST(K,L+4)*V(J)
      DO 210 J=1,12
210  ST(K,J)=PP1(J)
      DO 220 I=1,4
      XX=XM(I)*RHM
      DO 220 I=1,4
      P1(I,L)=P1(I,L)+XX*FMII(I,3)
      P1(I+4,L)=P1(I+4,L)+XX*FMII(I,4)
220  P1(I+8,L)=P1(I+8,L)+XX*FMII(I,5)
      DO 600 I=1,4
      DO 600 I=1,NS
600  T1(I,L)=T1(I,4)*FMII(I,J)
      PETHRM
      ENDO

```

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      PL AN3420
      PL AN3430
      PL AN3440
      PL AN3450
      PL AN3460
      PL AN3470
      PL AN3480
      PL AN3490
      PL AN3500
      PL AN3510
      PL AN3520
      PL AN3530
      PL AN3540
      PL AN3550
      PL AN3560
      PL AN3570

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      DO 200 ,J=1,3
      L1=4*(J-1)+1
200  PP1(L1)=S[(K,L1)*H1(J)+ST(K,L+4)*V(J)]
      DO 210 ,J=1,12
210  ST(K,J)=PP1(J)
      DO 220 I=1,4
      XX=XM(I)*RHO
      DO 220 L=1,4
      P1(I,L)=P1(I,L)+XX*FM11(L,3)
      P1(I+4,L)=P1(I+4,L)+XX*FM11(L,4)
220  P1(I+8,L)=P1(I+8,L)+XX*FM11(L,5)
      DO 600 L=1,4
      DO 600 I=1,N5
600  TT(I,L)=TT(I,4)*FM11(L,J)
      PFTUPEM
      ENDO

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PL AN3420
PI AN3430
PL AN3440
PI AN3450
PL AN3460
PI AN3470
PL AN3480
PI AN3490
PL AN3500
PI AN3510
PL AN3520
PI AN3530
PL AN3540
PI AN3550
PL AN3560
PI AN3570

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      SUBROUTINE PLANC2(S,R ,U,V,W)
C*****PI AN3580
C-----COORDINATE TRANSFORMATION OF STIFFNESS MATRIX FOR MEMBRANE ELEMENTPI AN3590
C*****PI AN3600
      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION S(12,12),R (12,12),U(4),V(4),W(4)
      DO 190 I=1,3
      DO 190 K=1,4
      KK=4*(I-1)+K
      DO 190 J=1,3
      DO 190 L=1,4
      LL=4*(J-1)+L
190 R(KK,LL)=U(I)*(S(K ,L)*U(J)+S(K ,L+4)*V(J))+
1 V(I)*(S(K+4,L)*U(J)+S(K+4,L+4)*V(J))+W(I)*S(K+8,L+8)*W(J)
      DO 196 I=1,12
      DO 196 J=1,12
      S(I,J)=R(I,J)
196 S(J,I)=S(I,J)
      RETURN
      END
      PI AN3620
      PI AN3630
      PI AN3640
      PI AN3650
      PI AN3660
      PI AN3670
      PI AN3680
      PI AN3690
      PI AN3700
      PI AN3710
      PI AN3720
      PI AN3730
      PI AN3740
      PI AN3750
      PI AN3760
      PI AN3770

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      SUBROUTINE PDSJNV(A)
C*****PI AN3780
C*****PI AN3790
C*****PI AN3800
C*****PI AN3810
      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION A(3,3)
      DO 200 N=1,3
      D=A(N,N)
      DO 100 J=1,3
100 A(N,J)=-A(N,J)/D
      DO 150 I=1,3
      IF(N-I) 110,150,110
110 DO 140 J=1,3
      IF(N-J) 120,140,120
120 A(I,J)=A(I,J)+A(I,N)*A(N,J)
140 CONTINUE
150 A(I,N)=A(I,N)/D
      A(N,N)=1./D
200 CONTINUE
      RETURN
      END
      PI AN3820
      PI AN3830
      PI AN3840
      PI AN3850
      PI AN3860
      PI AN3870
      PI AN3880
      PI AN3890
      PI AN3900
      PI AN3910
      PI AN3920
      PI AN3930
      PI AN3940
      PI AN3950
      PI AN3960
      PI AN3970
      PI AN3980

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      SHAPETINE FORMH(S,T,R)
C*****
C-----FORM SHAPE-FUNCTION DERIVATIVES AND STRAIN-DISPLACEMENT MATRIX
C*****
      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION R(12,12),II(6),JJ(6)
      COMMON/JUNK/IF(4),IX(4),FMUL(4,5),D(3,3),X(4),Y(4),Z(4),TMP(4)
      1 ALP(3),TII(3),PRESS,REF1,NS,MSG(3),RR(4),ZZ(4),PPI(12),
      2 UI(4),VI(4),WI(4),HI(6),HR(6),HZ(6),XJ,HS(6),HT(6),JUN(124)
      DATA 11/1,2,3,4,5,10/,JJ/5,6,7,8,11,12/
      SM=1,0-S
      SP=1,0+S
      TM=1,0-T
      TP=1,0+T
      H(1)=SM*TM*.25
      H(2)=SP*TM*.25
      H(3)=SP*TP*.25
      H(4)=SM*TP*.25
      H(5)=(1,0-S*S)
      H(6)=(1,0-T*T)
      HS(1)=-TM*.25
      HS(2)=-HS(1)
      HS(3)=TP*.25
      HS(4)=-HS(3)
      HS(5)=-2.*S
      HS(6)=0,0
      HT(1)=-SM*.25
      HT(2)=-SP*.25
      HT(3)=-HT(2)
      HT(4)=-HT(1)
      HT(5)=0,0
      HT(6)=-2.*T
      P7T=HT(3)*ZZ(3)+HT(4)*ZZ(4)
      PZS=HS(3)*ZZ(3)+HS(4)*ZZ(4)
      PRS=HS(2)*RR(2)+HS(3)*RR(3)+HS(4)*RR(4)
      PRT=HT(2)*RR(2)+HT(3)*RR(3)+HT(4)*RR(4)
      IF(DABS(ZZ(3)-ZZ(4)).LE.1,0F-10) PZS=0.
      IF(DABS(RR(2)-RR(3)).LE.1,0F-10.AND.DABS(RR(4)).LE.1,0F-10) PRT=0.
      XJ=PRS*P7T-PRT*PZS
      PSR=P7T/XJ
      PTR=-PZS/XJ
      PS7=-PRT/XJ
      PTZ=PRS/XJ
      DO 100 I=1,6
      HR(I)=PSR*HS(I)+PTR*HT(I)
      100 H7(I)=PS7*HS(I)+PTZ*HT(I)
C*****
C-----FORM STRAIN DISPLACEMENT MATRIX
C*****
      DO 200 K=1,6
      I=II(K)
      J=JJ(K)
      R(1,I)=HR(K)
      R(2,I)=H7(K)
      R(3,I)=H7(K)
      200 R(3,I)=HR(K)
      RETURN
      END

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PL AN3990
PL AN4000
PL AN4010
PL AN4020
PL AN4030
PL AN4040
PL AN4050
PL AN4060
PL AN4070
PL AN4080
PL AN4090
PL AN4100
PL AN4110
PL AN4120
PL AN4130
PL AN4140
PL AN4150
PL AN4160
PL AN4170
PL AN4180
PL AN4190
PL AN4200
PL AN4210
PL AN4220
PL AN4230
PL AN4240
PL AN4250
PL AN4260
PL AN4270
PL AN4280
PL AN4290
PL AN4300
PL AN4310
PL AN4320
PL AN4330
PL AN4340
PL AN4350
PL AN4360
PL AN4370
PL AN4380
PL AN4390
PL AN4400
PL AN4410
PL AN4420
PL AN4430
PL AN4440
PL AN4450
PL AN4460
PL AN4470
PL AN4480
PL AN4490
PL AN4500
PL AN4510
PL AN4520
PL AN4530
PL AN4540
PL AN4550
PL AN4560

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      SIARRDII IMF PI,NAXI(IHWI,IJ,X,Y,7,T,NTC,WI,PGFO,PMAT,NIIMDV,NIIMNP, PI AN4570
      1 NIIMMAT,NIIMTC,KODE,NIIME,NIIMGFO) PI AN4580
C*****PI AN4590
C-----SIIFFENFI MEMRANF FIFMENT PI AN4600
C*****PI AN4610
      IMPLTC IT REAI,8 (A-H,O-Z) PI AN4620
      REAI,8 IHWI,X,Y,7,T,PGFO,WI,PMAT,RFI,F4,F5,F6,F7,SBC,PBC,GHC,FRC PI AN4630
      DIMENS ION IHWI(NIIMDV),IO(NIIMNP,6),X(NIIMNP),Y(NIIMNP),Z(NIIMNP), PI AN4640
      1 I(NIIMNP),NTC(NIIMMAT),WI(NIIMMAT),PGFO(NIIMMAT,5), PI AN4650
      2 PMAT(NIIMTC,8,NIIMMAT) PI AN4660
      COMMON/FM/IM(12),S(12,12),PI(12,4),P2(12,4),XM(12),SI(15,12), PI AN4670
      1 TT(15,4),G(12,12,3),RR(12,12),FM(1700) PI AN4680
      COMMON/IIINK/IF(4),IX(4),FMUL(4,5),C(3,3),XX(4),YY(4),ZZ(4),IMP(4), PI AN4690
      1 ALP(3),TTI(3),PRESS,REFI,NS,NSG(3),X1,X2,X3,X4,Y1,Y2,Y3,Y4,FE(7), PI AN4700
      2 IIN(204) PI AN4710
      COMMON/CONTR/IC1(13),IRIICK,IC2(15) PI AN4720
      COMMON/UNITTS/IR,IW,P,I,12,13,IR,19,110,111,112,113 PI AN4730
C*****PI AN4740
C-----CONTRM INFORMATION PI AN4750
C*****PI AN4760
      MI=1 PI AN4770
      MV=1 PI AN4780
      MD=12 PI AN4790
      MW=1 PI AN4800
      MT=9 PI AN4810
      IFX=3 PI AN4820
      MC=3 PI AN4830
      DO 5 I=1,3 PI AN4840
5 MSG(I)=I PI AN4850
      WRITF(IW,2000)NIIME,KODE,NIIMMAT,NIIMGFO,NIIMTC PI AN4860
      WRITF(IW,2010) PI AN4870
      DO 60 M=1,NIIMMAT PI AN4880
      READ(IR,1010) N,NTC(N),WI(N) PI AN4890
      IF(NTC(N).LE.O) NTC(N)=1 PI AN4900
      WRITF(IW,2020) N,NTC(N),WI(N) PI AN4910
C*****PI AN4920
C-----TEMPERATURE DEPENDENT MATERIAL PROPERTIES PI AN4930
C*****PI AN4940
      NT=NTC(N) PI AN4950
      READ(IR,1005) (PMAT(I,J,N),J=1,8,I=1,NT) PI AN4960
      DO 10 J=1,NT PI AN4970
      IF(PMAT(I,6,N).LE.O) PMAT(I,6,N)=PMAT(I,5,N) PI AN4980
      IF(PMAT(I,7,NT).LE.O) PMAT(I,7,N)=PMAT(I,5,N)*O.577 PI AN4990
      IF(PMAT(I,8,NT).LE.O) PMAT(I,8,N)=PMAT(I,6,N) PI AN5000
      10 CONTINUE PI AN5010
      60 WRITF(IW,2010) (PMAT(I,J,N),J=1,8,I=1,NT) PI AN5020
C*****PI AN5030
C-----GEOMETRIC PROPERTY CARDS PI AN5040
C*****PI AN5050
      WRITE(IW,2011) PI AN5060
      DO 70 J=1,NIIMGFO PI AN5070
      READ(IR,1006) N,TH,W,SA,SI,I,WF PI AN5080
      IF(WF.LE.O) WF=W PI AN5090
      PGFO(N,1)=1.O+SA/(W*TH) PI AN5100
      PGFO(N,2)=W/TH PI AN5110
      PGFO(N,3)=WF/TH PI AN5120
      DR=SA*W/(W*TH+SA) PI AN5130
      RI=W*TH*3/12.O+W*TH*DR*DR*3/1+SA*(DR/PGFO(N,1))*2 PI AN5140
      PGFO(N,4)=RI/TH**4 PI AN5150
      CR=12.O*SI*O.925/(W*TH**3) PI AN5160

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      C2=SA*ND/SI                                PL AN5170
      C3=1.0+C2/(1.0+R8*PGFN(N,1)+0.12)          PL AN5180
      PGFN(N,5)=2.0*PGFN(N,2)**2*(DSOR1(1.0+C1*C3)+1.0) PL AN5190
70  WRITE(W,2012) N,TH,W,SA,SI,D,W,F            PL AN5200
C*****PI AN5210
C-----FLFMENT IJIAN MHIT IPI IFRS            PL AN5220
C*****PI AN5230
      DO 131 I=1,4                                PL AN5240
      READ(IR,1002) FMII(I,1),(FMII(I,J),J=3,5)    PL AN5250
131  FMII(I,2)=0.                                PL AN5260
      WRITE(W,2004) (FMII(I,1),(FMII(I,J),J=3,5) ,I=1,4) PL AN5270
C*****PI AN5280
C-----FLFMENT IARDS                            PL AN5290
C*****PI AN5300
      WRITE(W,2002)                                PL AN5310
      N=1                                           PL AN5320
120  READ(IR,1002) IFI,IF,IMAT,JDV,FRC,REFT,AA,AR,RETA,EFC,NS,INC PL AN5330
      IF (FRC,IFI,0) FRC=1.                        PL AN5340
      IF (EFC,IF,0) EFC=1.                          PL AN5350
      IF (INC,F0,0) INC=1                           PL AN5360
      IF (NS,F0,0) NS=3                              PL AN5370
      IF (NS,LT,3) NS=1                             PL AN5380
      IF ( (FI(3),F0,FI(4)) .AND. (NS,F0,15) ) NS=12 PL AN5390
      ANG=RETA/57.2957795                          PL AN5400
      RHO=W1/IMAT)                                PL AN5410
      THICK=PGFN(IMAT,1)                           PL AN5420
      RET=RETA)                                    PL AN5430
      KK=INC*(IFI-N)                                PL AN5440
      DO 142 J=1,4                                  PL AN5450
142  IX(J)=IF(J)-KK                                PL AN5460
      DO 500 NFI=N,IFI                              PL AN5470
      TFMP=0.                                         PL AN5480
      DO 501 J=1,4                                  PL AN5490
      IT=IX(J)                                       PL AN5500
      TFMP=TFMP+T(IT)*0.25                          PL AN5510
      XX(J)=X(IT)                                    PL AN5520
      YY(J)=Y(IT)                                    PL AN5530
      501  ZZ(J)=Z(IT)                              PL AN5540
C*****PI AN5550
C-----INTERPOLATE MATERIAL PROPERTIES FOR AVERAGE ELEMENT TEMPERATURE PL AN5560
C*****PI AN5570
      CALL INTERP(PMAT,FF,MIMIC,NIIMAT,R,7,NTC(IMAT),IMAT,TFMP) PL AN5580
      F4=FF(4)                                       PL AN5590
      F5=FF(5)                                       PL AN5600
      F6=FF(6)                                       PL AN5610
      F7=FF(7)                                       PL AN5620
      C1=FF(1)*0.8606                              PL AN5630
      C2=C1/(3.0*(1.0-FF(2)**2))                  PL AN5640
C*****PI AN5650
C-----FORM CONSTITUTIVE LAW AND COMPUTE THERMAL STRESSES PL AN5660
C*****PI AN5670
      DO 265 I=1,3                                  PL AN5680
      DO 265 J=1,3                                  PL AN5690
265  C(I,J)=0.                                       PL AN5700
      C(2,2)=1.0/FF(1)                             PL AN5710
      C(1,1)=C(2,2)/THICK                          PL AN5720
      E(1,2)=-C(1,1)*FF(2)                         PL AN5730
      C(2,1)=C(1,2)                                PL AN5740
      C(3,3)=C(2,2)*2.0*(1.0+FF(2))                PL AN5750
      ALP(1)=FF(3)                                  PL AN5760

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      ALP(2)=FF(3)
      ALP(3)=0.
      CALL FLAW(RH-TA)
C-----FORM ELEMENT LOCATION MATRIX AND COMPUTE ELEMENT MATRICES
C-----
      DO 170 I=1,4
      IT=IX(I)
      TMP(I)=T(I)
      LM(I)=ID(I,1)
      LM(I+4)=ID(I,2)
170  LM(I+8)=ID(I,3)
      CALL QUAD(RHO,THICK)
      ARFA=XM(1)+XM(2)+XM(3)+XM(4)
      IWT(I DV)=IWT(I DV)+ARFA*RHO*FRC
C-----COMPUTE ELEMENT DESIGN INFORMATION
C-----
      IF(AA,LF,0)AA=.5*(X2+X3-X1-X4)*DCOS(ANG)-(Y2+Y3-Y1-Y4)*DSIN(ANG)
      IF(AR,LF,0)AR=.5*(X3+X4-X1-X2)*DSIN(ANG)+(Y3+Y4-Y1-Y2)*DCOS(ANG)
      SRC=C1*FFC*PGFN(IMAT,4)/(AA*AA*PGFN(IMAT,2))
      PRC=C2*THICK/PGFN(IMAT,3)**2
      GRC=C2*0.25*PGFN(IMAT,5)/AR**4
C-----CALCULATE BANDWIDTH AND WRITE ELEMENT INFO. ON TAPES
C-----
      IF(NS,FO,15) GO TO 600
      NN=NS*ND*NI
      CALL REARRAN(ST,ST,15,12,1,NS,ND,NI,NN)
      NN=NS*4*NI
      CALL REARRAN(TT,TT,15,4,1,NS,4,NI,NN)
600  CALL CALRAN(INDIF,LM,S,P,ST,TT,NI,NV,NS,ND,NW,INDV,LEX,FRC)
      IF(LRUCK,NF,0) CALL FLRSHW(G,NSC,ND,NG,111)
      WRITE(IH,1)NI,RF1,F4,F5,F6,F7,PGFN(IMAT,1),SRC,PRC,GRC
      WRITE(IW,2003)NFI,IX,IMAT,INDV,FRC,RF1,AA,AR,BETA,FFC,NS,NDIF
      DO 450 I=1,4
450  IX(I)=IX(I)+INC
500  CONTINUE
      N=IFL+1
      IF(N,LF,NIUF) GO TO 130
      RETURN
1002 FORMAT(4F10.0)
1003 FORMAT(715.5X,4F10.0/2F10.0,215)
1005 FORMAT(8F10.0)
1006 FORMAT(15,6F10.0)
1010 FORMAT(215,F10.0)
2000 FORMAT(43H)NUMBER OF MEMBRANE ELEMENTS =,15/
      1 44H CONSTRUCTION CODE =,15/
      2 44H NUMBER OF MATERIALS =,15/
      3 44H NUMBER OF GEOMETRIC PROPERTIES =,15/
      4 44H NUMBER OF TEMPS FOR WHICH MATL PROPS GIVEN=,15)
2002 FORMAT(// 23H PROCESSED ELEMENT DATA//
      1121H ELEMNT/-----NODES-----//ID NOS-/ DES VAR REFERENCE
      2MAX LENGTH WIDTH ANGLE TO END FIXITY PRNT BAND /
      3121H NUMBR I J K L MAT DV FRACTION TEMP
      4OF STIFFNR OF ELEMNT PRINC DIRM COEFF1 COEF WIDTH /)
2003 FORMAT(1X,715.6F12.4,216)
2004 FORMAT(23H ELEMENT LOAD FRACTIONS /54H LOAD CASE TEMPERATURE X=0)
      1RECTION Y-DIRECTION Z-DIRECTION /4X,1H4.4F12.3/

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2 0X,1HR,4F12.3/ 9X,1HC,4F12.3/ 9X,1HD,4F12.3) PI AN6370
2010 FORMAT(1H+,25X,RE13.4/(26X,RE14.4)) PI AN6380
2011 FORMAT(191H GEOMETRY SHEET SPACING OF /-----STIFFEN PI AN6390
1ER PROPERTIES-----/ WIDTH OF / PI AN6400
2 01H NUMBER THICKNESS STIFFENERS AREA INERTIA PI AN6410
4 DIST OF CG SHEET ) PI AN6420
2012 FORMAT(1Y,15,6F14.4) PI AN6430
2013 FORMAT(// 25H MATERIAL PROPERTY CARDS / PI AN6440
1/125H MATL NO OF SPECIFIC YOUNGS POPI PI AN6450
255ONS COEFFT OF /-----ALLOWABLE STRESSES-----/ PI AN6460
3/121H NRR TEMP WEIGHT TEMPERATURE MODULUS RPI PI AN6470
4AT1D THERM EXPN TFNS COMP SHEAR CRIPLING /) PI AN6480
2020 FORMAT(1Y,14,16,2X,F14.4) PI AN6490
END PI AN6500

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SHARDUITIME DP1,AM1 (AD1D,ANFW,LOAD,NUMDV) PI AN6510
***** PI AN6520
C-----DESIGN OF STIFFENED MEMBRANE ELEMENT PI AN6530
***** PI AN6540
DIMENS1DN AD1D(NUMDV),ANFW(NUMDV),LOAD1(NUMDV) PI AN6550
COMMON/UNK/ LT,LH,L,SG(20),SIG(7),INVAR,IFX,FRC,ARFA, PI AN6560
1 XINFRT,RETA,TFNS,COMP,SHEAR,CRUSH,TAU,SRG,PHC,GRC,JUIN1(324) PI AN6570
PX=SIG(4) PI AN6580
PY=SIG(5) PI AN6590
PXY=SIG(6) PI AN6600
***** PI AN6610
C-----FINELY STRESSED DESIGN PI AN6620
***** PI AN6630
P1=COMP*TAU*ARFA PI AN6640
P2=COMP*ARFA PI AN6650
P12=SHEAR*ARFA PI AN6660
IF (PX.GT.0.0) P1=TFNS*TAU*ARFA PI AN6670
IF (PY.GT.0.0) P2=TFNS*ARFA PI AN6680
RMAX=(PX/P1)**2+(PY/P2)**2-(PX/P1)*(PY/P2)+(PXY/P12)**2 PI AN6690
RMAX=SQRT(RMAX) PI AN6700
IF (PX.GF.0.0) GO TO 100 PI AN6710
***** PI AN6720
C-----STIFFENER FAILURE PI AN6730
***** PI AN6740
P=-PX PI AN6750
PF=SRC*XINFRT PI AN6760
R=(P/PF)/(1.0/IFX) PI AN6770
IF (R.GT.RMAX) RMAX=R PI AN6780
P1=CRUSH*TAU*ARFA PI AN6790
P2=0.5*P1 PI AN6800
CALL JOHNS (IFX,P,P1,P2,PF,R) PI AN6810
IF (R.GT.RMAX) RMAX=R PI AN6820
***** PI AN6830
C-----SHEET BUCKLING BETWEEN STIFFENERS PI AN6840
***** PI AN6850
100 PX=-PX PI AN6860
PY=-PY PI AN6870
AA=PX+4.0*PY/TAU PI AN6880
RR=1.405*PXY/TAU PI AN6890
R=0.5*(AA+SQRT(AA**2+RR**2))/PHC PI AN6900
IF (P.GT.RMAX) RMAX=P PI AN6910

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C*****PI AN6920
C-----GENERAL BUICKLING OF PANELPL AN6930
C*****PI AN6940
      IF (PX.LT.O.O) GO TO 11RPL AN6950
      PF=GRG*XTNFRIT*AREA*ARFAPL AN6960
      AA=PX/PFPL AN6970
      R=AA**O.2PL AN6980
      IF (R.GT.RMAX) RMAX=RPL AN6990
11R AA=RMAX*ADID(I)VAR)PL AN7000
      IF (AA.LT.ANEW(I)VAR)) GO TO 60PL AN7010
      ANEW(I)VAR)=AAPL AN7020
      LADN(I)VAR)=LPL AN7030
60 CONTINUEPL AN7040
      RETIRNPL AN7050
      ENDPPL AN7060

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      SHROU17JNF PLMAX2(IW1,1D,X,Y,Z,I,NTC,W1,PMAT,NUMDV,NUMNP,NUMMAT), PI AN7070
      I NUMTC,KODF,NUMF) PI AN7080
C*****PI AN7090
C-----PI ANF ISOTROPIC MEMBRANE ELEMENTS PI AN7100
C*****PI AN7110
      IMPLCIT REAL*8 (A-H,O-Z) PI AN7120
      REAL*8 IW1,X,Y,Z,I,W1,PMAT,RF1,F4,F5,F6,FRC PI AN7130
      DIMENS(NDN,IWT(NUMDV),ID(NUMNP,6),X(NUMNP),Y(NUMNP),Z(NUMNP),
      IT(NUMNP),NTC(NUMMAT),W1(NUMMAT),PMAT(NUMTC,7,NUMMAT) PI AN7140
      COMMON/FM/LM(12),S(12,12),P(12,4,2),XM(12),ST(15,12),TT(15,4), PI AN7160
      I G(12,12,3),RR(12,12),FM1(1700) PI AN7170
      COMMON/JUNK/IF(4),IX(4),FMII(4,5),O(3,3),XX(4),YY(4),ZZ(4),TMP(4), PI AN7180
      I ALP(3),I1(13),PRESS,REF1,NS,NSG(3),FF(6),WIN(238) PI AN7190
      COMMON/CONTR/IC1(13),LNUCK,IC2(15) PI AN7200
      COMMON/UNIT5/IR,IW,IP,I1,I2,I3,IR,I9,I10,I11,I12,I13 PI AN7210
C*****PI AN7220
C-----CONTRL INFORMATION PI AN7230
C*****PI AN7240
      NI=1 PI AN7250
      ND=12 PI AN7260
      NV=2 PI AN7270
      NW=1 PI AN7280
      NI=4 PI AN7290
      NG=3 PI AN7300
      DO 5 J=1,3 PI AN7310
      5 NSG(I)=I PI AN7320
      IFX=0 PI AN7330
      WRITE(IW,2000)NUMF,KODF,NUMMAT,NUMTC PI AN7340
C*****PI AN7350
C-----MATRIAL PROPERTY CARDS PI AN7360
C*****PI AN7370
      WRITE(IW,2019) PI AN7380
      DO 60 M=1,NUMMAT PI AN7390
      READ(IR,1010) N,NTC(N),WT(N) PI AN7400
      IF(NTC(N).LE.0) NTC(N)=1 PI AN7410
      WRITE(IW,2020) N,NTC(N),WT(N) PI AN7420
C*****PI AN7430
C-----TEMPERATURE DEPENDENT MATERIAL PROPERTIES PI AN7440
C*****PI AN7450
      NT=NTC(N) PI AN7460
      READ(IR,1005) I(PMAT(I,J,N),J=1,7),I=1,NT) PI AN7470
      DO 10 J=1,NT PI AN7480
      IF(PMAT(I,6,N).LE.0.) PMAT(I,6,N)=PMAT(I,5,N) PI AN7490
      10 CONTINUE PI AN7500
      60 WRITE(IW,2010) I(PMAT(I,J,N),J=1,7),I=1,NT) PI AN7510
C*****PI AN7520
C-----ELEMENT LADN MULTIPLIES PI AN7530
C*****PI AN7540
      READ(IR,1002) I(FMII(I,J),J=1,5),I=1,4) PI AN7550
      WRITE(IW,2004) I(FMII(I,J),J=1,5),I=1,4) PI AN7560
C*****PI AN7570
C-----ELEMENT CARDS PI AN7580
C*****PI AN7590
      WRITE(IW,2002) PI AN7600
      N=1 PI AN7610
      130 READ(IR,1003) IFL,IF,IMAT,IDV,FRC,REF1,PRESS,RETA,NS,INC PI AN7620
      IF(FRC.LE.0.) FRC=1. PI AN7630
      IF(INC.F0.0) INC=1 PI AN7640
      IF(NS.F0.0) NS=3 PI AN7650
      IF(NS.LT.3) NS=1 PI AN7660

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      IF( (IF(3) .EQ. IF(4)) .AND. (NS.EQ. 15) ) NS=12          PL AN7670
      RHO=WT(IMAT)          PL AN7680
      RFT=RF1A             PL AN7690
      KK=INC*(IFL-N)        PL AN7700
      DO 142 I=1,4          PL AN7710
142  IX(I)=IF(I)-KK        PL AN7720
      DO 500 NFI=N,IFL     PL AN7730
      TFMP=0.               PL AN7740
      DO 501 I=1,4          PL AN7750
      TI=IX(I)              PL AN7760
      TFMP=TFMP+T(I)*0.25  PL AN7770
      XX(I)=X(TI)           PL AN7780
      YY(I)=Y(TI)           PL AN7790
501  ZZ(I)=Z(TI)           PL AN7800
C*****PI AN7810
C-----INTERPOLATE MATERIAL PROPERTIES FOR AVERAGE ELEMENT TEMPERATURE PL AN7820
C*****PI AN7830
      CALL INTERP(PMAT,FF,NUMTC,NUMMAT,7,6,NTC(IMAT),IMAT,TFMP) PL AN7840
      F4=FF(4)              PL AN7850
      F5=FF(5)              PL AN7860
      F6=FF(6)              PL AN7870
C*****PI AN7880
C-----FORM CONSTITUTIVE LAW AND COMPUTE THERMAL STRESSES PL AN7890
C*****PI AN7900
      DO 265 I=1,3          PL AN7910
      DO 265 J=1,3          PL AN7920
265  D(I,J)=0.             PL AN7930
      D(2,2)=1.0/FF(1)     PL AN7940
      D(1,1)=D(2,2)        PL AN7950
      D(1,2)=-D(1,1)*FF(2) PL AN7960
      D(2,1)=D(1,2)        PL AN7970
      D(3,3)=D(2,2)*2.0*(1.0+FF(2)) PL AN7980
      ALP(1)=FF(3)          PL AN7990
      ALP(2)=FF(3)          PL AN8000
      ALP(3)=0.             PL AN8010
      CALL FLAW(0.000)      PL AN8020
C*****PI AN8030
C-----FORM ELEMENT LOCATION MATRIX AND COMPUTE ELEMENT MATRICES PL AN8040
C*****PI AN8050
      DO 170 I=1,4          PL AN8060
      TI=IX(I)              PL AN8070
      TMP(I)=T(TI)          PL AN8080
      LM(I)=JD(TI,1)        PL AN8090
      LM(I+4)=ID(TI,2)      PL AN8100
170  LM(I+8)=ID(TI,3)       PL AN8110
      CALL QUAD(RHO, 1.000) PL AN8120
      AREA=XM(1)+XM(2)+XM(3)+XM(4) PL AN8130
      IJMT(IDV)=IJMT(IDV)+AREA*RHO*FRG PL AN8140
      IF(NS.EQ.15) GO TO 600 PL AN8150
      NN=NS*ND*NI          PL AN8160
      CALL REPARAN(ST,ST,15,12,1,NS,ND,NI,NN) PL AN8170
      NN=NS*4*NW           PL AN8180
      CALL REPARAN(TT,TT,15,4,1,NS,4,NW,NN) PL AN8190
600  CALL CALRAN(NDI,LM,S,P,ST,TT,NI,NV,NS,ND,NW,IDV,IFX,FRG) PL AN8200
      IF(THICK.NE.0) CALL FLGSIW(G,NSG,ND,NG,11) PL AN8210
      WRITE(I8) NI,RFT,F4,F5,F6 PL AN8220
      WRITE(I8,2002) NFI,IX,IMAT,IDV,FRG,RFT,PRESS,RF1A,NS,ND,IF PL AN8230
      DO 450 I=1,4          PL AN8240
450  JX(I)=IX(I)+INC      PL AN8250
500  CONTINUE

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N=IFL+1
IF(N,LE,NUMF) GO TO 130
RETURN
1002 FORMAT(5F10.0)
1003 FORMAT(7F5.0,3F10.0,2I5)
1005 FORMAT(7F10.0)
1010 FORMAT(2I5, F10.0)
2000 FORMAT(44H)NUMBER OF MEMBRANE ELEMENTS      =,15 /
1      44H CONSTRUCTION CODE      =,15/
2      44H NUMBER OF MATERIALS      =,15/
3      44H NUMBER OF TEMPS FOR WHICH MATL PROPS GIVEN=,15)
2002 FORMAT(/ /23H PROCESSED ELEMENT DATA//
1 9TH ELEM1/-----NODES-----//--IN M1S--/   DFS VAR   REFERENCE
2      PRMT RAND +/
3 9TH NUMBR I      J      K      L      MAT DIV   FRACTION   TEMP
4PRESSURE      BETA      CODE WIDTH      /)
2003 FORMAT(1X,7I5,4F12.4,2I6)
2004 FORMAT(23H ELEMENT LOAD FRACTIONS//71H LOAD CASE TEMPERATURE PREP
1SSURE X-DIRECTION Y-DIRECTION Z-DIRECTION /
2 6X ,1HA ,3X ,5F12.3/ 6X ,1HB ,3X ,5F12.3/ 6X ,1HC ,3X ,5F12.3/
3 6X ,1HD ,3X ,5F12.3 )
2010 FORMAT(1H+,27X,7F14.4 /(28X,7F14.4))
2019 FORMAT(/ / 25H MATERIAL PROPERTY CARDS /
1/125H MATL NO OF SPECIFIC      YOUNGS      PD
2SSONS      COEFFT OF /-----ALLOWABLE STRESSES-----/
3/121H NBR TEMP WEIGHT TEMPERATURE MODULUS      RPL
4ATTN      THERM EXPN TENSION COMPRESSION      SHEAR/)
2020 FORMAT(1X,I4,I6,2X,F14.4)
END

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PI ANR270
PI ANR280
PI ANR290
PI ANR300
PI ANR310
PI ANR320
PI ANR330
PI ANR340
PI ANR350
PI ANR360
PI ANR370
PI ANR380
PI ANR390
PI ANR400
PI ANR410
PI ANR420
PI ANR430
PI ANR440
PI ANR450
PI ANR460
PI ANR470
PI ANR480
PI ANR490
PI ANR500
PI ANR510
PI ANR520
PI ANR530
PI ANR540
PI ANR550

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SUBROUTINE DPLAN2 (ADLD,ANFW,LOAD,NUMDV)
C*****
C-----STRESS DESIGN OF ISOTROPIC MEMBRANE ELEMENT
C*****
DIMENSION ADLD(NUMDV),ANFW(NUMDV),LOAD(NUMDV)
COMMON/JUNK/      LT,LH,L,SG(20),SIG(7),IDVAR,IFX,FRC,AREA,
1 XINER1,BETA,TENS,COMP,SHEAR,HIN(329)
CC=(SIG(1)+SIG(2))*0.5
RR=(SIG(1)-SIG(2))*0.5
CR=SORT(RR*RR+SIG(3)**2)
PX=CC+CR
PY=CC-CR
P1=COMP*AREA
P2=COMP*AREA
IF (PX,GT,0.0) P1=TENS *AREA
IF (PY,GT,0.0) P2=TENS*AREA
RMAX=(PX/P1)**2+(PY/P2)**2-(PX/P1)*(PY/P2)
RMAX=SORT(RMAX)
IF(SHEAR,FO,0.) GO TO 50
PXY=CR/(AREA*SHEAR)
IF(RMAX,LT,PXY) RMAX=PXY
50 AA=RMAX*ADLD(IDVAR)
IF(AA,LT,ANFW(IDVAR)) GO TO 60
ANFW(IDVAR)=AA
LOAD(IDVAR)=1
60 RETURN
END

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PI ANR560
PI ANR570
PI ANR580
PI ANR590
PI ANR600
PI ANR610
PI ANR620
PI ANR630
PI ANR640
PI ANR650
PI ANR660
PI ANR670
PI ANR680
PI ANR690
PI ANR700
PI ANR710
PI ANR720
PI ANR730
PI ANR740
PI ANR750
PI ANR760
PI ANR770
PI ANR780
PI ANR790
PI ANR800
PI ANR810
PI ANR820

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      SUBROUTINE SHEAR( A,MTOT)                                SHER0000
C*****SHEAR PANEL ELEMENTS                                SHER0010
C-----SHEAR PANEL ELEMENTS                                SHER0020
C*****SHEAR PANEL ELEMENTS                                SHER0030
      DIMENSION A(MTOT)                                        SHER0040
      COMMON /FIPAR/ NPAR(14),NIMNP,MRAND,MFLTP,N1,N2,N3,N4,N5,M111,NEO$SHER0050
      L,NIMFL,NIMDV,M1,M2,M3,LL,LR,NFOR,NR,NCK              SHER0060
      COMMON /JINK/ LT,LH,L,SIG(27),IDVAR,IFX,FRC,APFA,XINERT,JIN(133) SHER0070
      COMMON /JINIS/ IR,TW,IP,II,I2,I3,IR,I9,I10,I11,I12,I13 SHER0080
      NIMF=NPARR(2)                                           SHER0090
      KODF=NPARR(5)                                           SHER0100
      IF(NPAR(1),F0,0)GO TO 500                               SHER0110
      NA=N5+NIMNP                                             SHER0120
      GO TO (1,2),KODF                                         SHER0130
C*****SHEAR PANEL WITH STRESS AND BUCKLING CONSTRAINTS SHER0140
C-----SHEAR PANEL WITH STRESS AND BUCKLING CONSTRAINTS SHER0150
C*****SHEAR PANEL WITH STRESS AND BUCKLING CONSTRAINTS SHER0160
      NIMMAT=NPARR(3)                                         SHER0170
      NIMTC=NPARR(4)                                           SHER0180
      N7=NA+NIMMAT                                             SHER0190
      NR=N7+NIMMAT                                             SHER0200
      NS=NR+NIMMAT*NIMTC*4                                     SHER0210
      MM=NO-MTOT                                                SHER0220
      IF(MM.GT.0)CALL FRRPR(MM)                                SHER0230
      CALL PANEL(A(M1),A(N1),A(N2),A(N3),A(N4),A(N5),A(N6),A(N7), SHER0240
      J A(NR),NIMDV,NIMNP,NIMMAT,NIMTC,KODF,NIMF)           SHER0250
      RETURN                                                    SHER0260
C*****SHEAR PANEL WITH STRESS AND BUCKLING CONSTRAINTS SHER0270
C-----PROVISION FOR SPECIAL SHEAR PANEL ELEMENT          SHER0280
C*****PROVISION FOR SPECIAL SHEAR PANEL ELEMENT          SHER0290
      CALL NDFLEM (NPAR(1),NPAR(5),TW)                        SHER0300
      RETURN                                                    SHER0310
      500 WRITE (1W,2002) KODF                                SHER0320
      DO 800 MM=1,NIMF                                          SHER0330
      CALL STRESS(A(M1),A(N1),A(N3),NFO,NIMDV,LL,LR,0)        SHER0340
      AA=A(IDVAR)*FRC                                           SHER0350
      WRITE (1W,2005) MM,AA                                     SHER0360
      DO 800 L=LT,LH                                           SHER0370
      IF(L.GT.LT) WRITE(1W,2006)                                SHER0380
      CALL STRESS(A(M1),A(N1),A(N3),NFO,NIMDV,LL,LR,1)        SHER0390
      SIG(5) = (SIG(1)+SIG(2)+SIG(3)+SIG(4)) *0.25           SHER0400
      WRITE (1W,2007) L,(SIG(I),I=1,5)                         SHER0410
      IF(L.NF,LH)WRITE (1W,2006)                                SHER0420
      GO TO (3,4),KODF                                           SHER0430
C*****PROVISION FOR SPECIAL SHEAR PANEL ELEMENT          SHER0440
C-----DESIGN OF SHEAR PANEL WITH STRESS AND BUCKLING CONSTRAINTS SHER0450
C*****DESIGN OF SHEAR PANEL WITH STRESS AND BUCKLING CONSTRAINTS SHER0460
      CALL DPANEL (A(M1),A(M2),A(M3),NIMDV)                  SHER0470
      GO TO 800                                                  SHER0480
C*****DESIGN OF SHEAR PANEL WITH STRESS AND BUCKLING CONSTRAINTS SHER0490
C-----PROVISION FOR DESIGN OF SPECIAL SHEAR PANEL ELEMENT SHER0500
C*****PROVISION FOR DESIGN OF SPECIAL SHEAR PANEL ELEMENT SHER0510
      4 CONTINUE                                                SHER0520
      800 CONTINUE                                              SHER0530
      RETURN                                                    SHER0540
      2002 FORMAT(//40H ANALYSIS OF SHEAR PANELS, CONSNP CODE=,I2 // SHER0550
      1 92H                                     LOAD /-----SHEAR FLOW AT NO$SHER0560
      2DES-----/ AVERAGE /                                SHER0570
      3 92H ELEMENT THICKNESS /CONO I J                     SHER0580
      4K I SHEAR FLOW /)                                       SHER0590

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2005 FORMAT(16,1X,F15.4)                                SHFR0600
2006 FORMAT(/)                                             SHFR0610
2007 FORMAT(1H+,23X,15,1X,5F12.4)                       SHFR0620
END                                                         SHFR0630

SUBROUTINE PANEL(IWT,ID,X,Y,Z,T,NTC,W1,PMAT,NUMDV,NUMNP,NUMMAT, SHFR0640
1 NUMTC,KODF,NIMF)                                         SHFR0650
C*****SHFR0660
C-----SHEAR PANEL ELEMENTS                               SHFR0670
C*****SHFR0680
      IMPLC=1 REAL*8 (A-H,O-Z)                             SHFR0690
      REAL*4 IWT,X,Y,Z,T,WT,PMAT,FRG,F3,SHCR              SHFR0700
      DIMENSION IWT(NUMDV),ID(NUMNP,6),X(NUMNP),Y(NUMNP), SHFR0710
      1 T(NUMNP),NTC(NUMMAT),WT (NUMMAT),PMAT(NUMTC,4,NUMMAT).CC(6,2) SHFR0720
      COMMON/EM/LM(12),S(12,12),P(12,4),ST(4,12),T1(4,4),XM(12), SHFR0730
      1 G(12,12),FM1(2356)                                SHFR0740
      COMMON/JUNK/FMUL(3,4),IF(4),IX(4),XX(4),YY(4),ZZ(4),FF(3),AREA, SHFR0750
      1 TF(4,2),U(4),V(4),Q(4),D(4),P1,P2 ,JUN1(252)     SHFR0760
      COMMON/CONTR/IC1(13),LBLOCK,IC2(15)                 SHFR0770
      COMMON/UNITS/IR,IW,IP,I1,I2,I3,I4,I5,I6,I7,I8,I9,I10,I11,I12,I13 SHFR0780
      DATA CC/5.35 , 8.99 , 8.99 , 5.35 , 5.35 , 7.07 , SHFR0790
      1 3.99 , 5.72 , 3.29 , 7.25 , 5.63 , 3.91 /        SHFR0800
C*****SHFR0810
C-----CONTROL INFORMATION                                SHFR0820
C*****SHFR0830
      NN=12                                                  SHFR0840
      NI=1                                                  SHFR0850
      NV=1                                                  SHFR0860
      NW=1                                                  SHFR0870
      NS=4                                                  SHFR0880
      NJ=2                                                  SHFR0890
      IFX=3                                                  SHFR0900
      NG=1                                                  SHFR0910
      NSG=1                                                 SHFR0920
      WRITE(IW,2000)NIMF,KODF,NUMMAT,NUMTC                SHFR0930
C*****SHFR0940
C-----MATERIAL PROPERTY CARDS                             SHFR0950
C*****SHFR0960
      WRITE(IW,2001)                                         SHFR0970
      DO 5 M=1,NUMMAT                                         SHFR0980
      READ(IR,1001) N,NTC(N),WT(N)                          SHFR0990
      IF (NTC(N).EQ.0) NTC(N)=1                             SHFR1000
      WRITE(IW,2002) N,NTC(N),WT(N)                        SHFR1010
C*****SHFR1020
C-----TEMPERATURE DEPENDENT MATERIAL PROPERTIES          SHFR1030
C*****SHFR1040
      NT=NTC(N)                                              SHFR1050
      DO 5 J=1,NT                                             SHFR1060
      READ(JP,5001) (PMAT(J,K,N),K=1,4)                     SHFR1070
      IF (J.NE.1) WRITE (IW,6002)                          SHFR1080
      5 WRITE (IW,6003) (PMAT(J,K,N),K=1,4)                 SHFR1090
C*****SHFR1100
C-----ELEMENT LOAD MULTIPLIERS                            SHFR1110
C*****SHFR1120
      READ(JR,1003) (FMUL(I,J),J=1,4),I=1,2)              SHFR1130
      WRITE(IW,2003) (FMUL(I,J),J=1,4),I=1,3)              SHFR1140
C*****SHFR1150
C-----ELEMENT CARDS                                       SHFR1160
C*****SHFR1170

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      WRITE(IW,2005)
      N=1
      6 READ(IR,1004) IFI,IF,IMAT,IPV,ISU,FRC,AL,BL,INC
      IF(INC.F0.0) INC=1
      IF(FRC.F0.0) FRC=1.0
      IF(IMAT.F0.0) IMAT=1
      RHO=WT(IMAT)
      KK=JMC*(IFI,-N)
      DO 50 I=1,4
      50 IX(I)=IFI(I)-KK
      DO 500 NFL=N,IFI
      IFMP=0.
      DO 100 I=1,4
      II=IX(I)
      XX(I)=X(II)
      YY(I)=Y(II)
      ZZ(I)=Z(II)
      100 IFMP=IFMP+I(II)*0.25
      C*****
      C-----INTERPOLATE MATERIAL PROPERTIES FOR AVERAGE ELEMENT TEMPERATURE
      C*****
      CALL INTERP(PMAT,FF,NIMTC,NIMMAT,4,3,NTC(IMAT),IMAT,IFMP)
      F3=FF(3)
      C*****
      C-----FORM ELEMENT INIT MATRICES AND LOAD VECTORS
      C*****
      SMOO=0.5*FF(1)/(1.0+FF(2))
      CALL SPANFL(SMOO,FF(2),RHO,XL,YL,NFL,IW)
      IF(LBUCK.F0.0) GO TO 343
      CALL SPGEOM(G)
      CALL FLGSJW(G,NSG,ND,NG,III)
      C*****
      C-----COMPUTE BUCKLING DATA
      C*****
      343 SHCR=0.
      IF(ISU.F0.0) GO TO 121
      IF(XL.GF.YL) GO TO 120
      H=YI
      YL=XL
      XL=H
      120 IF(AL.F0.) AL=XL
      IF(BL.F0.) BL=YL
      H=CC(ISU,1)+CC(ISU,2)*BL*BL/(AL*AL)
      SHCR=H*0.8696*FF(1)/(12.0*BI*BI*(1.0+FF(2)*FF(2)))
      121 INT(IPV)=INT(IPV)+RHO*AREA*FRC
      C*****
      C-----FORM LOCATION MATRIX AND COMPUTE BAND WIDTH
      C*****
      DO 470 I=1,4
      II=IX(I)
      DO 470 J=1,3
      IJ=(I-1)*3+J
      470 LM(IJ)=IN(II,I)
      CALL CALRAN(MD)F,LM,S,P,ST,TI,NI,NV,NS,ND,NW,IPV,IFX,FRC)
      WRITE(IR) NI,F3,SHCR
      WRITE(IW,2004) NFL,IX,IMAT,IPV,ISU,FRC,AL,BL,MDIF
      C*****
      C-----CHECK FOR MORE ELEMENTS
      C*****
      DO 450 I=1,4

```

```

      SHFR1180
      SHFR1190
      SHFR1200
      SHFR1210
      SHFR1220
      SHFR1230
      SHFR1240
      SHFR1250
      SHFR1260
      SHFR1270
      SHFR1280
      SHFR1290
      SHFR1300
      SHFR1310
      SHFR1320
      SHFR1330
      SHFR1340
      SHFR1350
      SHFR1360
      SHFR1370
      SHFR1380
      SHFR1390
      SHFR1400
      SHFR1410
      SHFR1420
      SHFR1430
      SHFR1440
      SHFR1450
      SHFR1460
      SHFR1470
      SHFR1480
      SHFR1490
      SHFR1500
      SHFR1510
      SHFR1520
      SHFR1530
      SHFR1540
      SHFR1550
      SHFR1560
      SHFR1570
      SHFR1580
      SHFR1590
      SHFR1600
      SHFR1610
      SHFR1620
      SHFR1630
      SHFR1640
      SHFR1650
      SHFR1660
      SHFR1670
      SHFR1680
      SHFR1690
      SHFR1700
      SHFR1710
      SHFR1720
      SHFR1730
      SHFR1740
      SHFR1750
      SHFR1760
      SHFR1770

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450 IX(I)=IX(I)+INC SHFR1780
500 CONTINUE SHFR1790
    N=IFL+1 SHFR1800
    IF(N,IF,NIME) GO TO 6 SHFR1810
    RETURN SHFR1820
1001 FORMAT(2I5,F10.0) SHFR1830
1003 FORMAT(4F10.0) SHFR1840
1004 FORMAT(8I5,3F10.0,I5) SHFR1850
2000 FORMAT(44H)NUMBER OF SHEAR PANEL ELEMENTS =,15/ SHFR1860
    1 44H CONSTRUCTION CODE =,15/ SHFR1870
    2 44H NUMBER OF MATERIALS =,15/ SHFR1880
    4 44H NUMBER OF TEMPS FOR WHICH MATL PROPS GIVEN=,15) SHFR1890
2001 FORMAT(// 25H MATERIAL PROPERTY CARDS // SHFR1900
101H MATERIAL NUMBER SPECIFIC YOUNGS POISSONSHFR1910
    2 ALLOWABLE / SHFR1920
301H NUMBER OF TEMPS WEIGHT TEMP MODULUS RATIO SHFR1930
    4 SHEAR /) SHFR1940
2002 FORMAT(16,5X,15,F12.4) SHFR1950
2003 FORMAT(// 25H ELEMENT LOAD MULTIPLIERS //20X,1HA,14X,1HB,14X,1HC, SHFR1960
    1 14X,1HD,/6H X-DIR,4F15.6/ 6H Y-DIR,4F15.6/ 6H Z-DIR,4F15.6 ) SHFR1970
2004 FORMAT(17,2X,4)16,3)17,3F12.4,16) SHFR1980
2005 FORMAT(// 23H PROCESSED ELEMENT DATA// SHFR1990
196H ELEMENT /-----NODE NOS-----//---EL ID NOS---/ BOUND DES VSHFR2000
2AR /-----FEECT PANEL DIMNS---/ RAND / SHFR2010
396H NUMBER I J K L MATL D VAR CODE FRACTISHFR2020
400 LONGER SHORTER WIDTH /) SHFR2030
5001 FORMAT(4F10.0) SHFR2040
6002 FORMAT(//) SHFR2050
6003 FORMAT(1H+,30X,4F12.4) SHFR2060
    FMD SHFR2070

SUBROUTINE SPCEOM (G) SHFR2080
C***** SHFR2090
C-----COMPUTE UNIT GEOMETRIC STIFFNESS MATRICES SHFR2100
C***** SHFR2110
IMPLICIT REAL*8 (A-H,O-Z) SHFR2120
DIMENSJON G(12,12) SHFR2130
COMMON/JUNK/FMUL(3,4),IE(4),IX(4),XX(4),YY(4),ZZ(4),FF(3),ARFA, SHFR2140
1 TF(4,2),U(4),V(4),O(4),D(4),P1,P2,AJ1(3),AJ2(3),JUN1(240) SHFR2150
DO 10 I=1,3 SHFR2160
    AJ1(I)=-TF(I,1)*V(1)+TF(I,2)*U(1) SHFR2170
10 AJ2(I)=-TF(I,1)*V(2)+TF(I,2)*U(2) SHFR2180
    DO 15 J=1,12 SHFR2190
    DO 15 J=1,12 SHFR2200
15 G(I,J)=0. SHFR2210
    DO 20 I=1,3 SHFR2220
    DO 20 J=1,3 SHFR2230
    X1=(AJ1(I)*AJ1(J)+D(I)*D(J))*O(1)*P1/P2 SHFR2240
    X2=(AJ2(I)*AJ2(J)+D(I)*D(J))*O(2)*P1/P2 SHFR2250
    G(I,J)=X1 SHFR2260
    G(I,J+6)=-X1 SHFR2270
    G(J+6,I)=-X1 SHFR2280
    G(I+6,J)=X1 SHFR2290
    G(I+3,J+3)=X2 SHFR2300
    G(I+3,J+9)=-X2 SHFR2310
    G(J+9,I+3)=-X2 SHFR2320
20 G(I+9,J+9)=X2 SHFR2330
    RETURN SHFR2340
    FMD SHFR2350

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      SHARPENING PLAN (G,GG,RHO,XL,YL,MK, [W])
C*****
C-----FORM SHEAR PLAN, ELEMENT MATRIX CFS
C*****
      IMPLICIT REAL*8 (A-H,O-Z)
      COMMON/JOINT/FMH(3,4),TE(4),X(4),Y(4),Z(4),FF(3),AREA,
1      TE(4,2),U(4),V(4),W(4),P(4),P2,V01(4),V02(4),V12(4),
2      V41(4),V012(4),T1(3),T1(3),JHIN(200)
      COMMON/FE/IM(12),S(12,12),P(12,4),ST(4,12),TT(4,4),XM(12),
1      FMH(2500)
C*****
C-----JOINT VECTORS ALONG DIAGONALS, SIDES AND NORMAL TO THE MEAN PLANE
C*****
      CALL VECTOR (V01,X(1),Y(1),Z(1),X(3),Y(3),Z(3))
      CALL VECTOR (V02,X(2),Y(2),Z(2),X(4),Y(4),Z(4))
      CALL VECTOR (V12,X(1),Y(1),Z(1),X(2),Y(2),Z(2))
      CALL VECTOR (V41,X(4),Y(4),Z(4),X(1),Y(1),Z(1))
      CALL CROSS (V01,V02,W)
      AREA=0.5*V01(4)*V02(4)*W(4)
C*****
C-----FORM TRANSFORMATION MATRIX X TF
C*****
      MH=DOT(V12,W)
      DO 10 I=1,3
10      VP12(I)=(V12(I)-MH*W(I))*V12(4)
      VP12(4)=DOT(V12,W)*V12(1)+VP12(2)*V12(2)+VP12(3)*V12(3)
      DO 20 I=1,3
20      TF(I,1)=VP12(I)/VP12(4)
      CALL CROSS(D,TF,TF(1,2))
C*****
C-----COMPUTE ELEMENT CORNER COORDINATES IN LOCAL AXES SYSTEM
C*****
      X1=0.0
      Y1=0.0
      X2=VP12(4)
      Y2=0.0
      X3=DOT(TF,V01)*V01(4)
      Y3=DOT(TF(1,2),V01)*V01(4)
      X4=-DOT(TF,V41)*V41(4)
      Y4=-DOT(TF(1,2),V41)*V41(4)
      X1=0.5*(X2-X1+X3-X4)
      Y1=0.5*(Y3+Y4)
      X2=X3-Y3+X4/Y4
      Y2=X4-X2+(X2-X3)*Y4/Y3
      IF(Y3,LT,.01,OR,Y4,LT,.01,OR,X34,LT,.01,OR,X42,GT,.01) GO TO 2006
      GO TO 57
2006      WRITF (TW,2007) MK
      STOP
C*****
C-----TEST FOR PARALLEL SIDES
C*****
57      A1=ABS((Y3-Y4)/(X3-X4))
      A2=ABS((Y4-X2-Y3+Y4)/(X4-(X2-X2)+Y4+Y3))
      IF(A1,LE,.01,AND,A2,LE,.01) GO TO 40
      IF(A1,LE,.01) GO TO 20
      IF(A2,LE,.01) GO TO 35
      GO TO 45
C*****
C-----CASE WHEN SIDES 1 AND 3 ARE PARALLEL
C*****

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30 YP=X2*Y3*Y4/(Y3*X4-Y4*(X3-X2))
P1=YP-Y1
P2=YP-Y2
P3=YP-Y3
P4=YP-Y4
XP=X2*Y3*X4/(Y3*X4-Y4*(X3-X2))
AA=(X2-XP)/YP
CC=(X1-XP)/YP
H=P1*P2*ARFA/(P3*P4*2.0*G)
H=H+H*(AA*AA+AA*CC+CC*CC)/(1.5*(1.0+GG))
G0 T0 46
*****SHFR2960
*****SHFR2970
*****SHFR2980
*****SHFR2990
*****SHFR3000
*****SHFR3010
*****SHFR3020
*****SHFR3030
*****SHFR3040
*****SHFR3050
*****SHFR3060
C-----CASE WHEN SIFIS 2 AND 4 ARE PARALLEL *****SHFR3070
C-----CASE WHEN SIFIS 2 AND 4 ARE PARALLEL *****SHFR3080
*****SHFR3090
35 ND=-0.5*(X4/Y4+(X3-X2)/Y3)
X0=X4-(X3-X4)*Y4/(Y3-Y4)
AD=J.0/DSOR1(1.0+ND*ND)
P1=(X0-X1-Y1*ND)*AD
P2=(X0-X2-Y2*ND)*AD
P3=(X0-X3-Y3*ND)*AD
P4=(X0-X4-Y4*ND)*AD
RR=(X0-X4)*ND+Y4/(X0-X4-Y4*ND)
H=P1*P2*ARFA/(P3*P4*2.0*G)
H=H+H*(RR*RR+RR*ND+ND*ND)/(1.5*(1.0+GG))
G0 T0 46
*****SHFR3100
*****SHFR3110
*****SHFR3120
*****SHFR3130
*****SHFR3140
*****SHFR3150
*****SHFR3160
*****SHFR3170
*****SHFR3180
*****SHFR3190
*****SHFR3200
C-----PARALLEL PROGRAM CASE *****SHFR3210
C-----PARALLEL PROGRAM CASE *****SHFR3220
*****SHFR3230
40 P1=1.0
P2=1.0
P3=1.0
P4=1.0
ND=-0.5*(X4/Y4+(X3-X2)/Y3+(Y3-Y4)/(X3-X4))
H=0.5*ARFA*(1.0+2.0*ND*ND)/(1.0+GG)/G
G0 T0 46
*****SHFR3240
*****SHFR3250
*****SHFR3260
*****SHFR3270
*****SHFR3280
*****SHFR3290
*****SHFR3300
C-----CASE WHEN NO PARALLEL SIFIS ARE PRESENT *****SHFR3310
C-----CASE WHEN NO PARALLEL SIFIS ARE PRESENT *****SHFR3320
*****SHFR3330
45 X0=X4-(X3-X4)*Y4/(Y3-Y4)
XP=X2*X4*Y3/(Y3*X4-Y4*(X3-X2))
YP=X2*Y3*Y4/(Y3*X4-Y4*(X3-X2))
N1S=DSOR1((X0-XP)*(X0-XP)+YP*YP)
ND=(X0-XP)/YP
P1=YP*(X0-X1-Y1*ND)/N1S
P2=YP*(X0-X2-Y2*ND)/N1S
P3=YP*(X0-X3-Y3*ND)/N1S
P4=YP*(X0-X4-Y4*ND)/N1S
CC=ND1S/P1-ND
RR=N1S/P4-CC
AA=ND1S/P2-ND
F=(AA+RR+(AA**3+RR**3)/1.5+0.2*(AA**5+RR**5))*DLOG(DABS(AA+RR))
1+(CC*ND+(CC**3+ND**3)/1.5+0.2*(CC**5+ND**5))*DLOG(DABS(CC+ND))
2-(RR+CC+(RR**3+CC**3)/1.5+0.2*(RR**5+CC**5))*DLOG(DABS(RR+CC))
3-(ND+AA+(ND**3+AA**3)/1.5+0.2*(ND**5+AA**5))*DLOG(DABS(ND+AA))
4+0.1*(AA*AA-CC*CC)*(RR**3-ND**3)+(RR*RR-ND*ND)*(AA**3-CC**3)
5-0.2*(AA-CC)*(RR**4-ND**4)+(RR-ND)*(AA**4-CC**4)
F=F*P1*P2*P3*P4*0.5/(N1S*ND)
H=0.5*P1*P2*(ARFA+4.0*(F-ARFA/1.5)/(1.0+GG))/(P3*P4*G)
*****SHFR3340
*****SHFR3350
*****SHFR3360
*****SHFR3370
*****SHFR3380
*****SHFR3390
*****SHFR3400
*****SHFR3410
*****SHFR3420
*****SHFR3430
*****SHFR3440
*****SHFR3450
*****SHFR3460
*****SHFR3470
*****SHFR3480
*****SHFR3490
*****SHFR3500
*****SHFR3510
*****SHFR3520
*****SHFR3530
*****SHFR3540
*****SHFR3550
C-----NEVELOP UNIT SIFIFNESS MATHX

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C*****$HFR3560
46 DF13=DSORT(X3*X3+Y3*Y3)$HFR3570
DF24=DSORT((X4-X2)*(X4-X2)+Y4*Y4)$HFR3580
U(1)=X3/DF13$HFR3590
U(2)=(X4-X2)/DF24$HFR3600
V(1)=Y3/DF13$HFR3610
V(2)=Y4/DF24$HFR3620
DO 47 I=3,4$HFR3630
U(I)=U(I-2)$HFR3640
47 V(I)=V(I-2)$HFR3650
O(1)=-X2*Y4*DF13*0.5/(X4*Y3-X3*Y4)$HFR3660
O(2)=X2*Y3*DF24*0.5/(X4*Y3-X3*Y4-X2*(Y3-Y4))$HFR3670
O(3)=-O(1)$HFR3680
O(4)=-O(2)$HFR3690
DO 100 J=1,4$HFR3700
DO 100 J=1,4$HFR3710
O(J)=O(J)*O(J)*0.5/H$HFR3720
DO 150 L=1,3$HFR3730
TJ(L)=TF(L,1)*U(1)+TF(L,2)*V(1)$HFR3740
150 TJ(L)=U(1)*TF(L,1)+V(1)*TF(L,2)$HFR3750
DO 160 L=1,3$HFR3760
DO 160 NM=1,3$HFR3770
J=3*(J-1)+L$HFR3780
J=3*(J-1)+NM$HFR3790
160 S(T1,J)=TJ(L)*TJ(NM)*O(J)$HFR3800
100 CONTINUE$HFR3810
DO 180 L=1,12$HFR3820
DO 180 M=L,12$HFR3830
180 S(L,M)=S(M,1)$HFR3840
C*****$HFR3850
C-----DEVELOP UNIT FORCE (STRESS) RECOVERY MATRIX$HFR3860
C*****$HFR3870
DO 300 I=1,4$HFR3880
J=(I-1)*3$HFR3890
SM=-O(I)*0.5/H$HFR3900
ST(1,I+1)=SM*(U(1)*TF(1,1)+V(1)*TF(1,2))$HFR3910
ST(1,I+2)=SM*(U(1)*TF(2,1)+V(1)*TF(2,2))$HFR3920
300 ST(1,I+3)=SM*(U(1)*TF(3,1)+V(1)*TF(3,2))$HFR3930
DO 400 J=1,12$HFR3940
ST(J)=ST(1,J)$HFR3950
ST(2,J)=ST(J)*P1/P2$HFR3960
ST(3,J)=ST(J)*P1*P2/(P3*P3)$HFR3970
ST(4,J)=ST(J)*P1*P2/(P4*P4)$HFR3980
400 ST(1,J)=ST(J)*P2/P1$HFR3990
C*****$HFR4000
C-----GRAVITY AND INERTIA LOADS$HFR4010
C*****$HFR4020
A1=0.5*X2*Y4$HFR4030
A2=0.5*X2*Y3$HFR4040
A3=APFA-A1$HFR4050
A4=APFA-A2$HFR4060
WTT=RH0/3.0$HFR4070
F1=(A4+A1+A2)*WTT$HFR4080
F2=(A1+A2+A3)*WTT$HFR4090
F3=(A2+A3+A4)*WTT$HFR4100
F4=(A3+A4+A1)*WTT$HFR4110
DO 450 I=1,3$HFR4120
XM(I)=F1$HFR4130
XM(I+3)=F2$HFR4140
XM(I+6)=F3$HFR4150

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      XM(I+9)=F4                                SHEF4160
      DO 450 I=1,4                                SHEF4170
      HH=FMII(I,L)                                SHEF4180
      P(I,L)=HH*F1                                SHEF4190
      P(I+3,L)=HH*F2                                SHEF4200
      P(I+6,L)=HH*F3                                SHEF4210
450  P(I+9,L)=HH*F4                                SHEF4220
      DO 460 I=1,4                                SHEF4230
      DO 460 I=1,4                                SHEF4240
460  TT(I,L)=0.0                                SHEF4250
      RETURN                                        SHEF4260
2007 FORMAT(1X,'ONE OF THE INTERIOR ANGLES FOR SHEAR PANEL NO.=',I5, ' SHEF4270
1 IS GREATER THAN 180 DEGREES.')                SHEF4280
      END                                          SHEF4290

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      SUBROUTINE DPANEL(ADLD,ANFW,LDAD,NUMDV)      SHEF4300
C***** SHEF4310
C-----DESIGN OF SHEAR PANEL ELEMENTS          SHEF4320
C***** SHEF4330
      DIMENSION ADLD(NUMDV),ANFW(NUMDV),LDAD(NUMDV) SHEF4340
      COMMON/JUNK/ LT,LT+1,SG(27),IDVAR,IFX,FRC,AREA,XINERT, SHEF4350
      1 SHEAR,SHCR,JUN1(33)                        SHEF4360
C***** SHEF4370
C-----CHECK SHEAR STRESS                       SHEF4380
C***** SHEF4390
      SHFLW=ABS(SG(15))                             SHEF4400
      RMAX=SHFLW/(SHEAR*AREA)                       SHEF4410
C***** SHEF4420
C-----CHECK BUCKLING                           SHEF4430
C***** SHEF4440
      IF (SHCR.LE.0.0) GO TO 4                      SHEF4450
      R=SHFLW/(SHCR*XINERT)                         SHEF4460
      R=R**0.333333                                SHEF4470
      3 IF (RMAX.LT.R) RMAX=R                       SHEF4480
C***** SHEF4490
C-----FULLY STRESSED DESIGN                     SHEF4500
C***** SHEF4510
      4 AA=RMAX*ADLD(IDVAR)                         SHEF4520
      IF (AA.LT.ANFW(IDVAR)) GO TO 60              SHEF4530
      ANFW(IDVAR)=AA                                SHEF4540
      LDAD(IDVAR)=1                                SHEF4550
60  CONTINUE                                        SHEF4560
      RETURN                                        SHEF4570
      END                                          SHEF4580

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SUBROUTINE SHELL (A,MTOT)
C*****
C-----PLATE/SHELL ELEMENTS
C*****
DIMENSION A(MTOT)
COMMON /ELPAR/ NPAR(14),NUMNP,MRAND,NELLYP,N1,N2,N3,N4,N5,MTT,NFO
COMMON /NUMDV,M1,M2,M3,LL,LR,NFOR,NBLOCK
COMMON /JUNK/ IT,LP,L,SG(20),SIG(7),IDV,IFX,FRC,THICK,
1 XINERT,TEN,COMP,SHEAR,RFTA,JUNI(329)
COMMON /UNITS/IR,IV,II,I2,I3,IP,IG,IIO,I11,I12,I13
NUMF=NPAR(2)
KODF=NPAR(5)
IF(NPAR(1),F0,0) GO TO 500
NUMMAT=NPAR(3)
NUMTC=NPAR(4)
N6=N5+NUMNP
N7=N6+NUMMAT
NR=N7+NUMMAT
GO TO (1,2),KODF
C*****
C-----ISOTROPIC PLATE/SHELL ELEMENTS
C*****
1 NO=NR+NUMMAT*NUMTC*7
MM=NO-MTOT
IF(MM.GT.0) CALL FRRDP(MM)
CALL PLATE1(A(M1),A(N1),A(N2),A(N3),A(N4),A(N5),A(N6),A(N7),A(NR),
1 NUMDV,NUMNP,NUMF,NUMMAT,NUMTC,KODF)
RETURN
C*****
C-----ORTHOTROPIC PLATE/SHELL ELEMENTS
C*****
2 CALL NOFLFM(NPAR(1),KODF,IV)
RETURN
500 WRITE(IW,2002) KODF
DO 800 MM=1,NUMF
CALL STPSC(A(M1),A(N1),A(N3),NFO,NUMDV,LL,LR,0)
WRITE(IW,2001) MM,THICK
TFTA=RFTA/57.2957795
CR=COS(TFTA)
SR=SIN(TFTA)
CSR=CR*SR
CR=CR*CR
SR=SR*SR
DO 800 I=1,1,1,H
IF(L.GT.1) WRITE(IW,2004)
CALL STPSC(A(M1),A(N1),A(N3),NFO,NUMDV,LL,LR,1)
IF(RFTA.NE.0.) GO TO 20
DO 30 I=1,6
30 SIG(I)=SG(I)
GO TO 40
20 DO 10 I=1,4,3
C1=SG(I)*CR+SG(I+1)*SR
C2=2,0*SG(I+2)*CSR
SIG(I)=C1+C2
SIG(I+1)=C1-C2
10 SIG(I+2)=(-SG(I)+SG(I+1))*CSR+SG(I+2)*(CR-SR)
40 WRITE(IW,2003) I,(SIG(I),I=1,6)
GO TO (3,4),KODF
C*****
C-----DESIGN OF ISOTROPIC PLATE/SHELL ELEMENTS

```

SHE1 0000

SHE1 0010

SHE1 0020

SHE1 0030

SHE1 0040

SHE1 0050

SHE1 0060

SHE1 0070

SHE1 0080

SHE1 0090

SHE1 0100

SHE1 0110

SHE1 0120

SHE1 0130

SHE1 0140

SHE1 0150

SHE1 0160

SHE1 0170

SHE1 0180

SHE1 0190

SHE1 0200

SHE1 0210

SHE1 0220

SHE1 0230

SHE1 0240

SHE1 0250

SHE1 0260

SHE1 0270

SHE1 0280

SHE1 0290

SHE1 0300

SHE1 0310

SHE1 0320

SHE1 0330

SHE1 0340

SHE1 0350

SHE1 0360

SHE1 0370

SHE1 0380

SHE1 0390

SHE1 0400

SHE1 0410

SHE1 0420

SHE1 0430

SHE1 0440

SHE1 0450

SHE1 0460

SHE1 0470

SHE1 0480

SHE1 0490

SHE1 0500

SHE1 0510

SHE1 0520

SHE1 0530

SHE1 0540

SHE1 0550

SHE1 0560

SHE1 0570

SHE1 0580

SHE1 0590

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C*****SHEL 0600
3 CALL DSHFL1(A(M1),A(M2),A(M3),NUMDV ) SHEL 0610
GO TO 800 SHEL 0620
C*****SHEL 0630
C-----DESIGN OF ORTHOTROPIC SHELL ELEMENTS SHEL 0640
C*****SHEL 0650
4 CONTINUE SHEL 0660
800 CONTINUE SHEL 0670
RETURN SHEL 0680
C*****SHEL 0690
2001 FORMAT(1X,I7,F14.4) SHEL 0700
2002 FORMAT(//40H ANALYSIS OF PLATE/SHELL ELEMENTS ,CONSTN CODE =,I9//SHEL 0710
1 113H ELEMENT ELEMENT LOAD /-----MEMBRANE FOSHEL 0720
PROCS-----//-----BENDING/TWISTING MOMENTS-----/ / SHEL 0730
3 113H NUMBER THICKNESS COND NXX NYY SHEL 0740
4 NXY MXX MYY MXY / ) SHEL 0750
2003 FORMAT(1H+,20X,I7,6F14.4) SHEL 0760
2004 FORMAT(/) SHEL 0770
END SHEL 0780

SUBROUTINE PLATE1(IWT,ID,X,Y,Z,T,WT,NTC,PMAT,NUMDV,NUMNP,NUME, SHEL 0790
1 NUMMAT,NUMTC,KDDF) SHEL 0800
C*****SHEL 0810
C-----ISOTROPIC PLATE/SHELL ELEMENTS - C.A.FELIPPA'S SHELL ELEMENT SHEL 0820
C-----NOTE (1) PROGRAM INCLUDES TEMP. GRADIENT LOAD VECTORS AND STRESSES SHEL 0830
C----- (2) PROGRAM IS WRITTEN FOR GENERAL ORTHOTROPIC MAT. PROPERTIF SHEL 0840
C-----THESE ARE NOT USED IN THE PRESENT PROGRAM SHEL 0850
C*****SHEL 0860
IMPLICIT REAL*8 (A-H,O-7) SHEL 0870
REAL*4 IWT,X,Y,Z,T,WT,PMAT,FRG,F4,F5,F6,RF1 SHEL 0880
DIMENSION IWT(NUMDV),ID(NUMNP,6),X(NUMNP),Y(NUMNP),Z(NUMNP), SHEL 0890
1 T(NUMNP),WT(NUMMAT),NTC(NUMMAT),PMAT(NUMTC,7,NUMMAT) SHEL 0900
COMMON/ININK/ SHEL 0910
110(3,3),NFM,NTRI,IX(4),IF(4),PRESS,TEMP,NTEMP,EMUL(5,4),NSG(3),JU, SHEL 0920
2 RHO ,R1(30),R2(30),ST1(6),ST2(6),XX(5),YY(5),ZZ(5),CM(3,3), SHEL 0930
3 ALFA(3), FF(16),ARFA ,JUN1(56) SHEL 0940
COMMON/FM/LM(24),S(30,30,2),P(24,4,3),XM(24),ST(6,30,2),TT(6,4,2), SHEL 0950
1 FM1(248) SHEL 0960
COMMON/COMP1/A(3,4),R(3,4),T1(9,4),T2(9,4),T3(9,4),LOC(3,4) , SHEL 0970
1 COM(288) SHEL 0980
COMMON/CONTR/IC1(13),LBICK,IC2(15) SHEL 0990
COMMON/UNIT5/IR,IW,IP,II,I2,I3,IR,IR,IR,I10,I11,I12 ,I13 SHEL 1000
DIMENSION G(30,30,3),IPERM(4),SC(6,24) SHEL 1010
EQUIVALENCE (G,S),(SC,R1) SHEL 1020
DATA IPERM/2,3,4,1/ SHEL 1030
C*****SHEL 1040
C-----CONTROL INFORMATION SHEL 1050
C*****SHEL 1060
MI=2 SHEL 1070
MV=2 SHEL 1080
MW=1 SHEL 1090
NS=6 SHEL 1100
NI=4 SHEL 1110
IFX=3 SHEL 1120
MG=3 SHEL 1130
DO 5 I=1,3 SHEL 1140

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5 MSG(1)=1 SHFL 1150
  NTEMP =0. SHFL 1160
  WRITE (IW,2000) NIIME ,NIUMMAT,MUMTC,KODF SHFL 1170
C***** SHFL 1180
C-----READ AND PRINT OF MATERIAL PROPERTIES SHFL 1190
C***** SHFL 1200
  WRITE (IW,2001) SHFL 1210
  DO 10 M=1,MUMMAT SHFL 1220
    READ (IR,1000) N,NTC(N),WT(N) SHFL 1230
    IF (NTC(N).EQ.0) NTC(N)=1 SHFL 1240
    WRITE (IW,2002) N,NTC(N),WT(N) SHFL 1250
    NT=NTC(N) SHFL 1260
    DO 11 I=1,NT SHFL 1270
      READ (IR,1003) (PMAT(I,J,N),J=1,7) SHFL 1280
      IF (PMAT(I,6,N).LE.0.) PMAT(I,6,N)=PMAT(I,5,N) SHFL 1290
      IF (PMAT(I,7,N).LE.0.) PMAT(I,7,N)=PMAT(I,5,N)*0.577 SHFL 1300
11 CONTINUE SHFL 1310
      WRITE (IW,2004) (PMAT(I,J,N),J=1,7) SHFL 1320
      IF (NT.GT.1) WRITE (IW,2008) (PMAT(I,J,N),J=1,7),I=2,NT SHFL 1330
10 CONTINUE SHFL 1340
C***** SHFL 1350
C-----READ AND PRINT OF ELEMENT LOAD MULTIPLIERS SHFL 1360
C***** SHFL 1370
  WRITE (IW,2006) SHFL 1380
  READ (IR,1002) (FMUL(I,J),J=1,4),I=1,5 SHFL 1390
  WRITE (IW,2007) (I,(FMUL(I,J),J=1,4),I=1,5) SHFL 1400
C***** SHFL 1410
C-----READ AND PRINT OF ELEMENT DATA SHFL 1420
C***** SHFL 1430
  WRITE (IW,2003) SHFL 1440
  N=1 SHFL 1450
100 READ (IR,1001) IFL,IF,IMAT,INC,IND,PRESS,REFT,FRC,BETA SHFL 1460
  IF (IFL.LT.N) GO TO 600 SHFL 1470
  IF (INC.EQ.0) INC=1 SHFL 1480
  IF (FRC.EQ.0.) FRC=1. SHFL 1490
  IF (IMAT.EQ.0) IMAT=1 SHFL 1500
  NFN=4 SHFL 1510
  ND=24 SHFL 1520
  NTR=4 SHFL 1530
  N3=5 SHFL 1540
  IF (IF(4).NE.0) GO TO 46 SHFL 1550
  NFN=3 SHFL 1560
  ND=18 SHFL 1570
  NTR=1 SHFL 1580
  N3=3 SHFL 1590
  IX(4)=0 SHFL 1600
46 RHN=WT(IMAT) SHFL 1610
  RET=RETA SHFL 1620
  KK=INC*(IFL-N) SHFL 1640
  DO 45 I=1,NFN SHFL 1640
45 IX(I)=IF(I)-KK SHFL 1650
  DO 500 NFI=N,IFL SHFL 1660
  TEMP=0. SHFL 1670
  DO 40 I=1,NFN SHFL 1680
  J=IX(I) SHFL 1690
  TEMP=TEMP+T(I) SHFL 1700
  XX(I)=X(I) SHFL 1710
  YY(I)=Y(I) SHFL 1720
40 Z(I)=Z(I) SHFL 1730
  TEMP=TEMP/NFN SHFL 1740

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      IF(NFN,NF,4) GO TO 75                                SHFL 1750
      XX(5)=0.25*(XX(1)+XX(2)+XX(3)+XX(4))                SHFL 1760
      YY(5)=0.25*(YY(1)+YY(2)+YY(3)+YY(4))                SHFL 1770
      ZZ(5)=0.25*(ZZ(1)+ZZ(2)+ZZ(3)+ZZ(4))                SHFL 1780
C*****SHFL 1790
C-----INTERPOLATE MATERIAL PROPERTIES FOR AVERAGE ELEMENT TEMPERATURE SHFL 1800
C*****SHFL 1810
      75 CALL INTERP (PMAT,EF,NUMTC,NUMMAT,7,6,NTC(IMAT),IMAT,TEMP) SHFL 1820
      TEMP=TEMP-REFT                                         SHFL 1830
      ALFA(1)=FF(2)                                         SHFL 1840
      ALFA(2)=FE(3)                                         SHFL 1850
      ALFA(3)=0.                                             SHFL 1860
      CONN=FF(1)/(1.0-FF(2)*FE(2))                         SHFL 1870
      CM(1,1)=CONN                                           SHFL 1880
      CM(1,2)=CONN*FF(2)                                     SHFL 1890
      CM(2,1)=CM(1,2)                                       SHFL 1900
      CM(2,2)=CONN                                           SHFL 1910
      CM(3,3)=FF(1)*0.5/(1.0+FF(2))                         SHFL 1920
      CM(1,3)=0.                                             SHFL 1930
      CM(2,3)=0.                                             SHFL 1940
      CM(3,1)=0.                                             SHFL 1950
      CM(3,2)=0.                                             SHFL 1960
      F4=FF(4)                                               SHFL 1970
      F5=FF(5)                                               SHFL 1980
      F6=FF(6)                                               SHFL 1990
C*****SHFL 2000
C-----COMPUTE DIRECTION COSINE MATRIX TO OF LOCAL ELEMENT SYSTEM SHFL 2010
C*****SHFL 2020
      CALL DDCOS (NTRI,XX,YY,ZZ,TD)                         SHFL 2030
C*****SHFL 2040
C-----COMPUTE DIRECTION COSINES OF LOCAL TRIANGLE SYSTEM SHFL 2050
C      AND THE TRIANGLE PROJECTIONS A,B ONTO IT SHFL 2060
C*****SHFL 2070
      DO 700 I=1,NTRI                                        SHFL 2080
      NI=I                                                    SHFL 2090
      N2=IPERM(NI)                                           SHFL 2100
      LDC(1,I)=N1*6-6                                         SHFL 2110
      LDC(2,I)= N2*6-6                                         SHFL 2120
      LDC(3,I)=N3*6-6                                         SHFL 2130
      700 CALL DDCOS(N1,N2,N3,XX,YY,ZZ,A(1,I),B(1,I),T1(1,I),T2(1,I),T3(1,I) SHFL 2140
      1,TD,NTRI)                                              SHFL 2150
C*****SHFL 2160
C-----FORM SHELL GLOBAL STIFFNESS MATRIX, MASS MATRIX, STRESS/ DISPLACEMENTS SHFL 2170
C-----FORM SHELL ELEMENT MATRICES SHFL 2180
C*****SHFL 2190
      AREA=0.                                                 SHFL 2200
      CALL DTSHEL (ND,NS)                                     SHFL 2210
C*****SHFL 2220
C-----FORM LM ARRAY SHFL 2230
C*****SHFL 2240
      DO 410 I=1,NFN                                         SHFL 2250
      J=6*I-6                                                 SHFL 2260
      L=IX(I)                                                 SHFL 2270
      DO 410 K=1,6                                             SHFL 2280
      410 LM(J+K)=TD(L,K)                                     SHFL 2290
      IWT(TDV)=IWT(TDV)+AREA*BHO*FPC                         SHFL 2300
      TF(LBCHK,NF,0,AND,NTRI,FD,4) CALL SH2221 (SC,SC(1,1,2)) SHFL 2310
C*****SHFL 2320
C-----COMPUTE RAN) WITH AND WRITE ELEMENT INFO. ON TAPES SHFL 2330
C*****SHFL 2340

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NN=ND*ND*NII                                SHEF.2350
CALL RFARANIS,S,30,30,2,ND,ND,NII,NN)        SHEF.2360
NN=NS*ND*NII                                SHEF.2370
CALL RFARAN(ST,ST,6,30,2,NS,ND,NII,NN)        SHEF.2380
NN=ND*4*NV                                  SHEF.2390
CALL RFARANIP,P,24,4,3,ND,4,NV,NN)           SHEF.2400
CALL CALRAN(NDIF,I,M,S,P,ST,TT,NII,NV,NS,ND,NW,IDV,IFX,FRC) SHEF.2410
WRITE(JR) MI,F4,F5,F6,RFI                     SHEF.2420
IF(LRICK.EQ.0) GO TO 650                       SHEF.2430
C*****SHEF.2440
C-----COMPUTE UNIT GEOMETRIC STIFFNESS MATRICES SHEF.2450
C*****SHEF.2460
CALL SHELG((SC,TD,NTRI))                      SHEF.2470
NN=ND*ND*NG                                    SHEF.2480
CALL RFARAN(G,G,30,30,3,ND,ND,NG,NN)          SHEF.2490
CALL FLGSIW(G,NSG,ND,NG,111)                 SHEF.2500
650 WRITE(IW,2004) NFI,IX,IMAT,IDV,PRESS,RFI,FRC,BETA,NDIF SHEF.2510
DO 450 MM=1,NFN                                SHEF.2520
450 IX(MM)=IX(MM)+INC                           SHEF.2530
500 CONTINUE                                    SHEF.2540
N=NFI+1                                         SHEF.2550
IF(N,LE,NIME) GO TO 100                       SHEF.2560
RETURN                                          SHEF.2570
600 WRITE(IW,2005) N                           SHEF.2580
STOP                                           SHEF.2590
1000 FORMAT(2I5,F10.0)                        SHEF.2600
1001 FORMAT(8I5,4F10.0)                      SHEF.2610
1002 FORMAT(4F10.0)                          SHEF.2620
1003 FORMAT(7F10.0)                          SHEF.2630
2000 FORMAT(50H)IT H I N P L A T E / S H E L L F L E M E N T S. // SHEF.2640
2      22H NUMBER OF ELEMENTS =, 15 /          SHEF.2650
3      22H NUMBER OF MATERIALS =, 15 /         SHEF.2660
4      22H NUMBER OF TEMP CARDS=, 15 /         SHEF.2670
5      22H CONSTR CODE =, 15 // )             SHEF.2680
2001 FORMAT(24H MATERIAL PROPERTY TABLE, // SHEF.2690
1      124H MATERIAL NUM OF SPECIFIC TEMP YOUNGS POSH) SHEF.2700
21SSONS'S COEFF OF /-----ALLOWABLE STRESSES-----SHEF.2710
3-/ / 117H NUMBER TEMP WEIGHT MODULUS SHEF.2720
4RATIO THERM EXPN TENSION COMPRESSION SHEAR /)SHEF.2730
2002 FORMAT(15,I9,F10.5)                     SHEF.2740
2003 FORMAT(132H THIN PLATE/SHELL ELEMENT DATA. // RH ELEMENT, 32X, SHEF.2750
1 8H MATERIAL,4X,7H DVS VAR,4X,6H NORMAL,4X,9H REFERENCE,5X,7H DVS VAR, SHEF.2760
.5X,4H BETA,8X,4H RAND / SHEF.2770
2 7H NUMBER,2X,6H NODE-I,2X,6H NODE-J,1X,6H NODE-K,2X,6H NODE-L, SHEF.2780
3 3X,6H NUMBER,5X,6H NUMBER,4X,8H PRESSURE,2X,11H TEMPERATURE, SHEF.2790
4 2X,8H REACTION,16X,5H WIDTH /) SHEF.2800
2004 FORMAT(15,4I8,2I9,3X,4F12.4,1I10)       SHEF.2810
2005 FORMAT(10HOCARD) FOR ELEMENT (,15,14H) IS IN ERROR., / 1X) SHEF.2820
2006 FORMAT(130H ELEMENT LOAD CASE MULTIPLIERS. // 13H ELEMENT LOAD, SHEF.2830
1 4X,8H PRESSURE,5X,7H THERMAL,13X,2HX-,13X,2HY-,13X,2HZ-, / SHEF.2840
2 13H CASE NUMBER,17X,7H EFFECTS, 3(3X,12H ACCELERATION), / 1X) SHEF.2850
2007 FORMAT(6X,1I,6X,2F12.3,3F15.3)         SHEF.2860
2008 FORMAT(26X,F10.3,1PF14.5,OPF10.3,1PF14.5,OPF14.2) SHEF.2870
2009 FORMAT(11H+,25X,F10.3,1PF14.5,OPF10.3,1PF14.5,OPF14.2) SHEF.2880
END                                             SHEF.2890

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      SUBROUTINE DDCOS (N,X,Y,Z)
C*****
C-----THIS SUBROUTINE COMPUTES THE DIRECTION COSINES OF THE LOCAL
C ELEMENT SYSTEM OF A QUADRILATERAL(N=4) OR SINGLE TRIANGLE(N=1)
C*****
      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION X(5),Y(5),Z(5),T(9)
      X1 = X(2)+X(3)-X(N)-X(1)
      Y1 = Y(2)+Y(3)-Y(N)-Y(1)
      Z1 = Z(2)+Z(3)-Z(N)-Z(1)
      X2 = X(3)+X(N)-X(1)-X(2)
      Y2 = Y(3)+Y(N)-Y(1)-Y(2)
      Z2 = Z(3)+Z(N)-Z(1)-Z(2)
      S1 = X1**2+Y1**2+Z1**2
      C = (X1*X2+Y1*Y2+Z1*Z2)/S1
      X2 = X2-C*X1
      Y2 = Y2-C*Y1
      Z2 = Z2-C*Z1
      S1=DSORT(S1)
      S2=DSORT(X2**2+Y2**2+Z2**2)
      X1=X1/S1
      Y1=Y1/S1
      Z1=Z1/S1
      X2=X2/S2
      Y2=Y2/S2
      Z2=Z2/S2
      T(1) = X1
      T(2) = X2
      T(3) = Y1*Z2-Y2*Z1
      T(4) = Y1
      T(5) = Y2
      T(6) = Z1*X2-Z2*X1
      T(7) = Z1
      T(8) = Z2
      T(9) = X1*Y2-X2*Y1
      RETURN
      END

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SHFL 2900  
SHFL 2910  
SHFL 2920  
SHFL 2930  
SHFL 2940  
SHFL 2950  
SHFL 2960  
SHFL 2970  
SHFL 2980  
SHFL 2990  
SHFL 3000  
SHFL 3010  
SHFL 3020  
SHFL 3030  
SHFL 3040  
SHFL 3050  
SHFL 3060  
SHFL 3070  
SHFL 3080  
SHFL 3090  
SHFL 3100  
SHFL 3110  
SHFL 3120  
SHFL 3130  
SHFL 3140  
SHFL 3150  
SHFL 3160  
SHFL 3170  
SHFL 3180  
SHFL 3190  
SHFL 3200  
SHFL 3210  
SHFL 3220  
SHFL 3230  
SHFL 3240  
SHFL 3250  
SHFL 3260

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      SUBROUTINE TDCOS(N1,N2,N3,X,Y,Z,A,R,T1,T2,T3,T,NTRI)
C*****
C-----THIS SUBROUTINE COMPUTES THE DIRECTION COSINES OF THE LOCAL
C SYSTEM AND THE PROJECTED DIMENSIONS OF A SUBTRIANGLE COMPONENT
C*****
      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION X(5),Y(5),Z(5),A(3),R(3),T1(9),T2(9),T3(9),T(9)
      A1 = X(N1)-X(N3)
      R1 = Y(N1)-Y(N3)
      C1 = Z(N1)-Z(N3)
      A2 = X(N2)-X(N3)
      R2 = Y(N2)-Y(N3)
      C2 = Z(N2)-Z(N3)
      IF(NTRI.FO.4) GO TO 300
      DO 350 J=1,3
      T1(J)=T1**2-2)
      T1(1+3)=T1(1)
      T1(1+6)=T1(1)
      T2(1)=T1(1*3-1)

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SHFL 3270  
SHFL 3280  
SHFL 3290  
SHFL 3300  
SHFL 3310  
SHFL 3320  
SHFL 3330  
SHFL 3340  
SHFL 3350  
SHFL 3360  
SHFL 3370  
SHFL 3380  
SHFL 3390  
SHFL 3400  
SHFL 3410  
SHFL 3420  
SHFL 3430  
SHFL 3440  
SHFL 3450



	T2(I+3)=T2(I)	SHFL 3460
	T2(I+6)=T2(I)	SHFL 3470
	T3(I)=T1*I*3	SHFL 3480
	T3(I+3)=T3(I)	SHFL 3490
350	T3(I+6)=T3(I)	SHFL 3500
	GO TO 400	SHFL 3510
300	T31 = R1*C2-R2*C1	SHFL 3520
	T32 = C1*A2-C2*A1	SHFL 3530
	T33 = A1*R2-A2*R1	SHFL 3540
	S = DSORT ( T31**2+T32**2+T33**2 )	SHFL 3550
	T31 = T31/S	SHFL 3560
	T32 = T32/S	SHFL 3570
	T33 = T33/S	SHFL 3580
	T11= T33*T(5)-T32*T(8)	SHFL 3590
	T12= T31*T(8)-T33*T(2)	SHFL 3600
	T13= T32*T(2)-T31*T(5)	SHFL 3610
	S = DSORT ( T11**2+T12**2+T13**2 )	SHFL 3620
	T11=T11/S	SHFL 3630
	T12=T12/S	SHFL 3640
	T13=T13/S	SHFL 3650
	T21= T13*T32-T12*T33	SHFL 3660
	T22= T11*T33-T13*T31	SHFL 3670
	T23=T12*T31-T11*T32	SHFL 3680
	T1(1)=T11	SHFL 3690
	T1(2)=T12	SHFL 3700
	T1(3)=T13	SHFL 3710
	T2(1)=T21	SHFL 3720
	T2(2)=T22	SHFL 3730
	T2(3)=T23	SHFL 3740
	T3(1)=T31	SHFL 3750
	T3(2)=T32	SHFL 3760
	T3(3)=T33	SHFL 3770
	DO J=1,3	SHFL 3780
	J=I+3	SHFL 3790
	K=I+6	SHFL 3800
	T1(J)=T1(I)	SHFL 3810
	T2(J)=T2(I)	SHFL 3820
	T3(J)=T3(I)	SHFL 3830
	C1=T1(I)	SHFL 3840
	C1=T(I)	SHFL 3850
	CK=T(K)	SHFL 3860
	T1(K)=T11*C1+T12*CJ+T13*CK	SHFL 3870
	T2(K)=T21*C1+T22*CJ+T23*CK	SHFL 3880
100	T3(K)=T31*C1+T32*CJ+T33*CK	SHFL 3890
400	A(1)= -T1(1)*A2-T1(2)*R2-T1(3)*C2	SHFL 3900
	A(2)= T1(1)*A1+T1(2)*R1+T1(3)*C1	SHFL 3910
	A(3)= -A(1)-A(2)	SHFL 3920
	R(1)= T2(1)*A2+T2(2)*R2+T2(3)*C2	SHFL 3930
	R(2)= -T2(1)*A1-T2(2)*R1-T2(3)*C1	SHFL 3940
	R(3)=-R(1)-R(2)	SHFL 3950
	RETURN	SHFL 3960
	END	SHFL 3970

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SUBROUTINE OTSHEL (ND,NS)                                SHEF 3980
C*****THIS SUBROUTINE EVALUATES                          SHEF 3990
C      STIFFNESS MATRIX                                    SHEF 4000
C      STRESS/DISPLACEMENT TRANSFORMATION MATRIX         SHEF 4010
C      NODAL FORCE VECTOR DUE TO DISTRIBUTED LATERAL LOADS SHEF 4020
C      NODAL FORCE VECTOR DUE TO THERMAL STRAINS          SHEF 4030
C      STRESS CORRECTION MATRIX DUE TO THERMAL STRAINS   SHEF 4040
C      AND MASS MATRIX                                    SHEF 4050
C      OF A SHALLOW QUADRILATERAL SHELL ELEMENT ASSEMBLED WITH FOUR FLAT SHEF 4060
C      TRIANGLES OR OF A SINGLE TRIANGULAR SHELL ELEMENT SHEF 4070
C-----S1 : UNIT STIFFNESS PROPORTIONAL TO THICKNESS    SHEF 4080
C      (DUE TO MEMBRANE ACTION)                          SHEF 4090
C-----S2 : UNIT STIFFNESS PROPORTIONAL TO (THICKNESS)**3 SHEF 4100
C      (DUE TO BENDING ACTION)                          SHEF 4110
C-----P1 : UNIT NODAL FORCE VECTOR PROPORTIONAL TO THICKNESS SHEF 4120
C      (DUE TO GRAVITY LOADS -POINT LOADS ONLY COMPUTED , SHEF 4130
C      AND DUE TO MEAN TEMPERATURE DIFFERENCE )         SHEF 4140
C-----P2 : UNIT NODAL FORCE VECTOR AND IS CONSTANT     SHEF 4150
C      (DUE TO NORMAL PRESSURE LOADS-LUMPED LOADS ONLY COMPUTED ) SHEF 4160
C-----P3 : UNIT NODAL FORCE VECTOR PROPORTIONAL TO (THICKNESS)**3 SHEF 4170
C      (DUE TO TEMPERATURE GRADIENT ACROSS THICKNESS)   SHEF 4180
C-----XM : MASS MATRIX PROPORTIONAL TO THICKNESS -LUMPED MASSES ONLY SHEF 4190
C-----SA1 : UNIT STRESS MATRIX PROPORTIONAL TO THICKNESS SHEF 4200
C      (DUE TO MEMBRANE ACTION)                          SHEF 4210
C-----SA2 : UNIT STRESS MATRIX PROPORTIONAL TO (THICKNESS)**3 SHEF 4220
C      (DUE TO BENDING ACTION)                          SHEF 4230
C-----T1 : UNIT STRESS CORRECTION VECTOR PROPORTIONAL TO THICKNESS SHEF 4240
C      (DUE TO MEAN TEMPERATURE DIFFERENCE - MEMBRANE STRESSES) SHEF 4250
C-----T2 : STRESS CORRECTION VECTOR PROPORTIONAL TO (THICKNESS)**3 SHEF 4260
C      (DUE TO TEMPERATURE GRADIENT ACROSS THICKNESS)   SHEF 4270
C*****SHELL 4280
C      IMPLICIT REAL*8 (A-H,O-Z)                        SHEF 4290
C      COMMON/SHK/                                       SHEF 4300
C      1 ND(3,3),NEN,NTR1,IX(4),IF(4),PRESS,TEMP,DTEMP,FMUL(5,4),NSG(3),JU, SHEF 4310
C      2 RHO ,R1(30),R2(30),ST1(6),ST2(6),X (5),Y (5),Z (5),CM(3,3), SHEF 4320
C      3 ALFA(3), FF(16),ARFA ,JUNI(56)                SHEF 4330
C      COMMON/CMPL/AL(3,4),R(3,4),T1(9,4),T2(9,4),T3(9,4),LOC(3,4) SHEF 4340
C      1,ARFA1,SM1(3),RM1(3) ,COM(281)                SHEF 4350
C      COMMON/FM/LM(24),S1(30,30),S2(30,30),P1(24,4),P2(24,4),P3(24,4), SHEF 4360
C      1XM(24),SA1(6,30),SA2(6,30),T11(6,4),T12(6,4),F1(9),C1(3,9),ST(9,9) SHEF 4370
C      1 ,FM1(131)                                     SHEF 4380
C      DIMENSION FMM(2700)                             SHEF 4390
C      EQUIVALENCE (FMM,S)                             SHEF 4400
C      WG=1.0                                           SHEF 4410
C      IF(NTR1,FO,4) WG=0.25                            SHEF 4420
C      DO 50 J=1,2700                                  SHEF 4430
C      50 FMM(I)=0.                                     SHEF 4440
C      DO 51 I=1,30                                    SHEF 4450
C      RI(I)=0.                                         SHEF 4460
C      51 R2(I)=0.                                     SHEF 4470
C      DO 52 I=1,4                                     SHEF 4480
C      52 R2(I)=0.                                     SHEF 4490
C-----THERMAL STRESS CORRECTION MATRIX                SHEF 4500
C*****SHELL 4510
C      DT=DTEMP/12.                                     SHEF 4520
C      DO 160 J=1,3                                     SHEF 4530
C      CC=CM(I,1)*ALFA(1)+CM(I,2)*ALFA(2)+CM(I,3)*ALFA(3) SHEF 4540
C      SM1(I) =-CC*TEMP                                SHEF 4550
C      RM1(I) =-CC*DT                                    SHEF 4560
C      DO 160 I=1,4                                     SHEF 4570

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      T11(I,J)=SMT(I)*FMIIL(2,J)
      160 T12(I+3,J)=RMT(I)*FMIIL(2,J)
C-----LOAD OVER THE NTRJ TRIANGLE COMPONENTS
C-----FORM MASS MATRIX AND NODAL FORCE VECTOR DUE TO NORMAL PRESSURE
C-----AND GRAVITY LOADS IN GLOBAL COORDINATES
      DO 700 NT = 1,NTRJ
      ARFAT=(A(2,NT)*R(2,NT)-A(2,NT)*H(3,NT))*0.5
      APFA=ARFAT*ARFAT
      IF(NTRJ.F0.) GO TO 345
      FAC=ARFAT*PRESS*0.5
      XMM=ARFAT*RH0*0.5
      DO 340 I=1,2
      K=LOC(I,NT)
      DO 340 J=1,3
      K=K+1
      DO 341 L=1,4
      P1(K,L)=P1(K,L)+XMM*FMIIL(J+2,L)
341 P2(K,L)=P2(K,L)+FAC*FMIIL(1,L)*T3(J,NT)
340 XM(K)=XM(K)+XMM
      GO TO 350
345 FAC=ARFAT*PRESS/3.
      XMM=ARFAT*RH0/3.
      DO 360 I=1,3
      K=LOC(I,NT)
      DO 360 J=1,3
      K=K+1
      DO 361 L=1,4
      P1(K,L)=P1(K,L)+XMM*FMIIL(J+2,L)
361 P2(K,L)=P2(K,L)+FAC*FMIIL(1,L)*T3(J,NT)
360 XM(K)=XM(K)+XMM
C-----MEMBRANE CONTRIBUTION
350 CALL SLST (CM,FT,CT,ST,NT)
C-----COORDINATE TRANSFORMATION OF TRIANGLE ELEMENT MEMBRANE STIFFNESS
      IT=0
      DO 400 JI=1,3
      J = JI + JI
      M = LOC(JI,NT)
      DO 400 I=1,3
      M = M + 1
      IT=IT+1
      C1=T1(IT,NT)
      C2=T2(IT,NT)
      K1=0
      DO 390 IT=1,JJ
      I = IT + IT
      KK=3
      IF (IT.F0.,JJ) KK = 1
      H1 = ST(I-1,J-1)*C1 + ST(I-1,J)*C2
      H2 = ST(I,J-1)*C1 + ST(I,J)*C2
      N = LOC(I,NT)
      DO 390 K=1,KK
      M = M + 1

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      SHEF 4580
      SHEF 4590
      SHEF 4600
      SHEF 4610
      SHEF 4620
      SHEF 4630
      SHEF 4640
      SHEF 4650
      SHEF 4660
      SHEF 4670
      SHEF 4680
      SHEF 4690
      SHEF 4700
      SHEF 4710
      SHEF 4720
      SHEF 4730
      SHEF 4740
      SHEF 4750
      SHEF 4760
      SHEF 4770
      SHEF 4780
      SHEF 4790
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      SHEF 4960
      SHEF 4970
      SHEF 4980
      SHEF 4990
      SHEF 5000
      SHEF 5010
      SHEF 5020
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      SHEF 5040
      SHEF 5050
      SHEF 5060
      SHEF 5070
      SHEF 5080
      SHEF 5090
      SHEF 5100
      SHEF 5110
      SHEF 5120
      SHEF 5130
      SHEF 5140
      SHEF 5150
      SHEF 5160
      SHEF 5170

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      KT=KT+1                                SHFL 5180
      S0= S1(N,M)+T1(KT,NT)*H1+T2(KT,NT)*H2    SHFL 5190
      S1(N,M)=S0                                SHFL 5200
390  S1(M,N)=S0                                SHFL 5210
400  CONTINUE                                  SHFL 5220
C*****SHFL 5230
C-----COORDINATE TRANSFORMATION OF STRESS MATRIX AND THERMAL LOAD VECTOR SHFL 5240
C*****SHFL 5250
      DO 410 J=1,3                                SHFL 5260
      M=LNC(J,J,NT)                                SHFL 5270
      J=J+1,J                                SHFL 5280
      DO 410 I=1,3                                SHFL 5290
      C1=T1(I,NT)                                SHFL 5300
      C2=T2(I,NT)                                SHFL 5310
      M = M+1                                SHFL 5320
      R1(M)=R1(M)+C1 *FT(J-1)+C2 *FT(J)          SHFL 5330
      DO 410 K=1,3                                SHFL 5340
      410  SA1(K,M)=SA1(K,M)+(C1(K,J-1)*C1 +C1(K,J)*C2 )*WG    SHFL 5350
C*****SHFL 5360
C-----PLATE BENDING CONTRIBUTION                                SHFL 5370
C*****SHFL 5380
      CALL SLCCCT (CM,FT,CT,ST,NT,NTRI)          SHFL 5390
C*****SHFL 5400
C-----COORDINATE TRANSFORMATION OF TRIANGLE ELEMENT BENDING STIFFNESS SHFL 5410
C*****SHFL 5420
      CALL SHLCT1(S2,ST,T1,T2,T3,LNC,NT)        SHFL 5430
C*****SHFL 5440
C-----COORDINATE TRANSFORMATION OF MOMENT RESULTANT MATRIX AND SHFL 5450
C THERMAL LOAD VECTOR                                SHFL 5460
C*****SHFL 5470
      DO 680 J=1,3                                SHFL 5480
      M=LNC(J,J,NT)                                SHFL 5490
      J=(J-1)*3+1                                SHFL 5500
      DO 686 I=1,3                                SHFL 5510
      M = M+1                                SHFL 5520
      C3=T3(I,NT)                                SHFL 5530
      R2(M)=R2(M)+FT(J)*C3                      SHFL 5540
      DO 686 K=1,3                                SHFL 5550
      686  SA2(K+3,M)=SA2(K+3,M)+C1(K,J)*C3*WG    SHFL 5560
      DO 680 I=1,3                                SHFL 5570
      M=M+1                                SHFL 5580
      C1=T1(I,NT)                                SHFL 5590
      C2=T2(I,NT)                                SHFL 5600
      R2(M)=R2(M)+FT(J+1)*C1 +FT(J+2)*C2        SHFL 5610
      DO 680 K=1,3                                SHFL 5620
      680  SA2(K+3,M)=SA2(K+3,M)+(C1(K,J+1)*C1+CT(K,J+2)*C2)*WG    SHFL 5630
700  CONTINUE                                  SHFL 5640
      IF(NTRI,FO,1) GO TO 900                    SHFL 5650
C*****SHFL 5660
C-----CHECK FOR POSSIBLE INTERNAL STIFFNESS SINGULARITY (FLAT SHFL 5670
C OR NEARLY FLAT QUADRILATERAL) AND TRANSFORM STIFFNESS AT 51TH NODE SHFL 5680
C TO GLOBAL COORDINATES                                SHFL 5690
C*****SHFL 5700
      IF(S1(27,27)+CT*(S1(25,25)+S1(26,26))*1.0E-07) GO TO 690    SHFL 5710
      DO 691 I=1,27                                SHFL 5720
      S1(I,27)=0.0                                SHFL 5730
      691  S1(27,I)=0.0                                SHFL 5740
      690  DO 510 II=1,27                            SHFL 5750
      DO 511 J=1,3                                SHFL 5760
      511  FT(J)=S1(II,25)*IP(1,J)+S1(II,26)*IP(2,J)+S1(II,27)*IP(3,J)    SHFL 5770

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      DO 510 J=1,3
510  S1(I,I,J+24)=F1(I,J)
      DO 520 J=25,27
      DO 521 I=1,3
521  F1(I)=T0(I,I)*S1(25,J)+T0(2,I)*S1(26,J)+T0(3,I)*S1(27,J)
      DO 520 I=1,3
520  S1(24+I,J)=F1(I)
      DO 530 I=1,24
      DO 530 J=25,27
530  S1(J,I)=S1(I,J)
      CALL SHLC72(S2,T0,F1,F1(4),F1(7))
C*****
C-----CONDENSATION OF INTERNAL DEGREES OF FREEDOM
C*****
      CALL SHLC01(1,3,S1,R1,SA1,ST1)
      CALL SHLC01(4,6,S2,R2,SA2,ST2)
      DO 851 J=1,6
      DO 851 J=1,4
      T1(I,J)=T1(I,J)+ST1(I)*FMUJ(2,J)
851  T1(I,J)=T1(I,J)+ST2(I)*FMUJ(2,J)
900  DO 850 J=1,NN
      DO 850 J=1,4
      P1(I,J)=P1(I,J)+R1(I)*FMUJ(2,J)
850  P3(I,J)=R2(I)*FMUJ(2,J)
      RETURN
      END
      SHFL 5780
      SHFL 5790
      SHFL 5800
      SHFL 5810
      SHFL 5820
      SHFL 5830
      SHFL 5840
      SHFL 5850
      SHFL 5860
      SHFL 5870
      SHFL 5880
      SHFL 5890
      SHFL 5900
      SHFL 5910
      SHFL 5920
      SHFL 5930
      SHFL 5940
      SHFL 5950
      SHFL 5960
      SHFL 5970
      SHFL 5980
      SHFL 5990
      SHFL 6000
      SHFL 6010
      SHFL 6020
      SHFL 6030

      SUBROUTINE SHLC01(NN,MM,S,R,SA,ST)
C*****
C-----CONDENSATION OF INTERNAL DEGREES OF FREEDOM
C*****
      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION S(30,30),R(30),SA(6,30),ST(6)
      DO 850 J=1,6
850  ST(I)=0.
      DO 800 J=1,6
      I=30-J
      M=I+1
      PIV=S(M,M)
      IF(PIV.EF.0) GO TO 800
      RI=R(M)/PIV
      DO 820 K=J+1
      R(K)=R(K)-S(K,M)*RI
      SS=S(M,K)/PIV
      DO 830 I=1,K
830  S(K,I)=S(K,I)-S(M,I)*SS
      DO 820 I=NN,MM
820  SA(I,K)=SA(I,K)-SA(I,M)*SS
      DO 810 I=NN,MM
810  ST(I)=ST(I)-SA(I,M)*RI
800  CONTINUE
      DO 900 I=2,24
      I1=I-1
      DO 900 J=1,J1
900  S(I,I)=S(I,I)
      RETURN
      END
      SHFL 6040
      SHFL 6050
      SHFL 6060
      SHFL 6070
      SHFL 6080
      SHFL 6090
      SHFL 6100
      SHFL 6110
      SHFL 6120
      SHFL 6130
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      SHFL 6180
      SHFL 6190
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      SHFL 6210
      SHFL 6220
      SHFL 6230
      SHFL 6240
      SHFL 6250
      SHFL 6260
      SHFL 6270
      SHFL 6280
      SHFL 6290
      SHFL 6300
      SHFL 6310
      SHFL 6320
      SHFL 6330

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SUBROUTINE SLSTIC,FT,CT,ST,NT)
C*****
C-----THIS SUBROUTINE FORMS THE STIFFNESS MATRIX ,THERMAL LOAD VECTOR,
C      AND STRESS MATRIX OF A CONSTANT STRAIN TRIANGLE
C*****
      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION C(3,3),FT(9),CT(3,9),ST(9,9)
      COMMON/COMMONPL/A(3,4),R(3,4),TT(108),LNC(3,4),ARFA ,SMT(3),RMT(3) ,
      L COM(281)
      FAC=0.25/ARFA
      FAC2=0.5/ARFA
      C11 = C(1,1)*FAC
      C22 = C(2,2)*FAC
      C33 = C(3,3)*FAC
      C12 = C(1,2)*FAC
      C13 = C(1,3)*FAC
      C23 = C(2,3)*FAC
      DO 200 J=1,3
      L=L+J
      A(J)=A(J,NT)
      R(J)=R(J,NT)
C*****
C-----THERMAL LOAD VECTOR
C*****
      FT(L-1)=(-R(J) *SMT(1)-A(J) *SMT(3))*0.5
      FT(L) =(-A(J) *SMT(2)-R(J) *SMT(3))*0.5
C*****
C-----STRESS DISPLACEMENT TRANSFORMATION MATRIX
C*****
      DO 300 I=1,3
      CT(I,L-1)= (C(I,1)*R(J) +C(I,3)*A(J) )*FAC1
      300 CT(I,L) = (C(I,2)*A(J) +C(I,3)*R(J) )*FAC1
C*****
C-----STIFFNESS MATRIX IN TRIANGLE LOCAL COORDINATES
C*****
      DO 200 I=1,J
      K=I+1
      AA=A(I,NT)*A(J)
      AB=A(I,NT)*R(J)
      BB=R(I,NT)*R(J)
      RA=R(I,NT)*A(J)
      ABA=AB+BA
      ST(K-1,L-1)=C11*BB+C13*ABA+C33*AA
      ST(K-1,L)=C12*BA+C13*BB+C23*AA+C33*AB
      ST(K,L-1)= C12*AB+C13*BB+C23*AA+C33*BA
      200 ST(K,L)= C22*AA+C23*ABA+C33*BB
      DO 400 J=3,6
      DO 400 J=1,I
      400 ST(J,J)=ST(I,J,I)
      RETURN
      END

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SHEF.6340

SHEF.6350

SHEF.6360

SHEF.6370

SHEF.6380

SHEF.6390

SHEF.6400

SHEF.6410

SHEF.6420

SHEF.6430

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SHEF.6450

SHEF.6460

SHEF.6470

SHEF.6480

SHEF.6490

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SHEF.6660

SHEF.6670

SHEF.6680

SHEF.6690

SHEF.6700

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SHEF.6720

SHEF.6730

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SHEF.6760

SHEF.6770

SHEF.6780

SHEF.6790

SHEF.6800

SHEF.6810

SHEF.6820

SHEF.6830

SHEF.6840

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SUBROUTINE SLCTT(CM,FT,CT,ST,NT,NTRI)
C*****
C-----FORM PLATE BENDING STIFFNESS AND LOAD VECTOR DUE TO
C THERMAL GRADIENT OF A LINEAR CURVATURE COMPATIBLE TRIANGLE(LCCT-9)
C NORMAL SLOPES AT MID SIDE NODES ARE ELIMINATED USING
C THETA(NN) AT NODE 4 = THETA(NN) AT NODE 1 + THETA(NN) AT NODE 2
C*****
IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION CM(3,3),F1(9),CT(3,9),ST(9,9),IPERM(3)
COMMON/COMPL/A(3,4),R(3,4),IT(108),LPC(3,4),AREA,SMI(3),RM1(3),
1 H(3),TX(3),TY(3),O1(3,6),P(21,9),G(21),HT(3),COM(41)
DATA IPERM/2,3,1/
FAC1=AREA/432.
FAC2=1./12.
DO 150 I=1,3
J = IPERM(I)
K = IPERM(J)
A1=A(I,NT)
A2=A(J,NT)
R1=R(I,NT)
R2=R(J,NT)
X=A1+A2+R1+R2
U(I)=- (A1*A2+R1*R2)/X
Y=DSORT(Y)
Y=4.*AREA/X
HT(I) = 2.*Y
TX(I) = Y*A1/X
TY(I) = -Y*R1/X
A1=0.5*A1 /AREA
A2=0.5*A2 /AREA
R1=0.5*R1 /AREA
R2=0.5*R2 /AREA
O(1,I) = H1*R1
O(2,I) = A1*A1
O(3,I) = 2.*A1*R1
O(1,I+2) = 2.*R1*R2
O(2,I+3) = 2.*A1*A2
O(3,I+3) = 2.*(A1*R2+A2*R1)
150 CONTINUE
C*****
C-----CURVATURE - DISPLACEMENT RELATIONS FOR 3 SUBTRIANGLE REGIONS
C*****
DO 200 I=1,3
J=IPERM(I)
K=IPERM(J)
IJ=3*I
J1=3*J
KK=3*K
A1=A(I,NT)
A2=A(J,NT)
A3=A(K,NT)
R1=R(I,NT)
R2=R(J,NT)
R3=R(K,NT)
U1=U(I)
U2=U(J)
U3=U(K)
W1=1.-U1
W2=1.-U2
W3=1.-U3

```

SHELL 6850  
 SHELL 6860  
 SHELL 6870  
 SHELL 6880  
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 SHELL 7390  
 SHELL 7400  
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 SHELL 7420  
 SHELL 7430  
 SHELL 7440

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R10=R1+R1
R20=R2+R2
R30=R3+R3
A10=A1+A1
A20=A2+A2
A30=A3+A3
C21 = R1-R3*U3      +TX(K)
C31 = A1-A3*U3      +TY(K)
C51 = R3*W3-R2      +TX(K)
C61 = A3*W3-A2      +TY(K)
C81 = R3-R20-R2*U2   +TX(J)
C91 = A3-A20-A2*U2   +TY(J)
C22=-R10+R2*W2+R3*U3 + TX(J)-TX(K)
C32=-A10+A2*W2+A3*U3+TY(J)-TY(K)
C52 = R20-R3*W3-R1*U1 +TX(J)-TX(K)
C62 = A20-A3*W3-A1*U1 +TY(J)-TY(K)
C82 = R10-R3+R1*W1   +TX(I)
C92 = A10-A3+A1*W1   +TY(I)
DO 200 N=1,3
L= 6*(I-1)+N
O11=O(N,I)
O22=O(N,I+1)
O33=O(N,K)
O12=O(N,I+3)
O23=O(N,I+3)
O31=O(N,K+3)
O233=O23-O33
O3133=O31-O33
P(L ,I1-2) = 6.*(O11+W2*O33+U3*O233)
P(L ,I1-1) = C21*O23+C22*O33-R20*O12+R20*O31
P(L ,I1 ) = C31*O23+C32*O33-A30*O12+A20*O31
P(L ,J1-2) = 6.*(O22+W3*O233)
P(L ,J1-1) = C51*O233+R30*O22
P(L ,J1 ) = C61*O233+A30*O22
P(L ,KK-2) = 6.*(I+U2)*O33
P(L ,KK-1) = C81*O33
P(L ,KK ) = C91*O33
P(L+3 ,J1-2) = 6.*(O11+U3*O3133)
P(L+3 ,I1-1) = C21*O3133-R30*O11
P(L+3 ,I1 ) = C31*O3133-A30*O11
P(L+3 ,J1-2) = 6.*(O22+U1*O33+W3*O3133)
P(L+3 ,J1-1) = C51*O31+C52*O33+R30*O12-R10*O23
P(L+3 ,J1 ) = C61*O31+C62*O33+A30*O12-A10*O23
P(L+3 ,KK-2) = 6.*(I+W1)*O33
P(L+3 ,KK-1) = C82*O33
P(L+3 ,KK ) = C92*O33
P(N+1R,I1-2) = 2.*(O11+U3*O12+W2*O31)
P(N+1R,KK-1) = (R10-R20)*O33+C82*O23+C81*O31)/2.
P(N+1R,KK ) = (A10-A20)*O33+C92*O23+C91*O31)/2.
200 CONTINUE
C*****SHELL 7450
C-----STIFFNESS MATRIX AND THERMAL LOAD VECTOR SHELL 7460
C*****SHELL 7470
DO 400 J=1,4
FT(J)=0.
DO 340 I=1,3
IT=I
KK=I+1P
P3=P(KK,I)
G(KK)=0.
SHELL 7480
SHELL 7490
SHELL 8000
SHELL 8010
SHELL 8020
SHELL 8030
SHELL 8040

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      SUBROUTINE SHLCT1(S2,S1,T1,T2,T3,LNC,NT)
C*****
C-----COORDINATE TRANSFORMATION OF TRIANGLE ELEMENT BENDING STIFFNESS
C*****
      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION T1(9,4),T2(9,4),T3(9,4),S2(30,30),LNC(3,4),S1(9,9)
      DO 500 JJ = 1,3
        JT = 3*JJ-3
        J = JT + 1
        DO 400 II = 1,3
          IT = 3*II-3
          I = IT + 1
          KK=6
          DO 400 L=1,6
            IF (IT.FO.,JJ), KK = L
            M = LNC(JJ,NT)+L
            L3 = L - 3
            IF (L3.GT.0) GO TO 460
            C3=T3(JT+L,NT)
            H1 = ST(I,J)*C3
            H2 = ST(I+1,J)*C3
            H3 = ST(I+2,J)*C3
            GO TO 470
          460 C1=T1(JT+L3,NT)
              C2=T2(JT+L3,NT)
              H1 = ST(I,J+1)*C1 + ST(I,J+2)*C2
              H2 = ST(I+1,J+1)*C1 + ST(I+1,J+2)*C2
              H3 = ST(I+2,J+1)*C1 + ST(I+2,J+2)*C2
          470 M = LNC(II,NT)
              DO 400 K = 1,KK
                N = M + 1
                K3 = K - 3
                K1 = IT + K
                K2 = IT + K3
                IF (K3.LE.0) S0 = S2(N,M) + T3(K,NT)*H1
                IF (K3.GT.0) S0 = S2(N,M) + T1(K2,NT)*H2+T2(K2,NT)*H3
                S2(N,M)= S0
          400 S2(M,N)= S0
        500 CONTINUE
      RETURN
      END

```

```

      SHFL A480
      SHFL A490
      SHFL A500
      SHFL A510
      SHFL A520
      SHFL A530
      SHFL A540
      SHFL A550
      SHFL A560
      SHFL A570
      SHFL A580
      SHFL A590
      SHFL A600
      SHFL A610
      SHFL A620
      SHFL A630
      SHFL A640
      SHFL A650
      SHFL A660
      SHFL A670
      SHFL A680
      SHFL A690
      SHFL A700
      SHFL A710
      SHFL A720
      SHFL A730
      SHFL A740
      SHFL A750
      SHFL A760
      SHFL A770
      SHFL A780
      SHFL A790
      SHFL A800
      SHFL A810
      SHFL A820
      SHFL A830
      SHFL A840
      SHFL A850
      SHFL A860
      SHFL A870
      SHFL A880

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```

      SUBROUTINE SHLCT2(S,T0,C1,C2,C3)
C*****
C-----TRANSFORM THE STIFFNESS MATRIX AT 5TH NODE TO GLOBAL COORDINATES
C*****
      IMPLICIT REAL*8 (A-H,O-Z)
      DIMENSION S(30,30),T0(3,3),C1(3),C2(3),C3(3)
      IF (S(25,25).GT. S(27,27)*1.0E-07) GO TO 692
      DO 693 I=1,30
        S(I,25)=0.0
      693 S(25,I)=0.0
      692 IF (S(26,26).GT. S(27,27)*1.0E-07) GO TO 694
      DO 695 I=1,30
        S(I,26)=0.0
      695 S(26,I)=0.0
      694 IF (S(30,30).GT. (S(29,29)+S(28,28))*1.0E-08) GO TO 730
      DO 710 I=1,30
        S(I,30)=0.0

```

```

      SHFL A890
      SHFL A900
      SHFL A910
      SHFL A920
      SHFL A930
      SHFL A940
      SHFL A950
      SHFL A960
      SHFL A970
      SHFL A980
      SHFL A990
      SHFL 9000
      SHFL 9010
      SHFL 9020
      SHFL 9030
      SHFL 9040
      SHFL 9050

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```

710 S(30,I)=0. SHFL 9060
720 DO 10 I=1,30 SHFL 9070
    DO 11 J=1,3 SHFL 9080
        C1(I)=S(11,25)*T0(1,I)+S(11,26)*T0(2,I)+S(11,27)*T0(3,I) SHFL 9090
    11 C2(I)=S(11,28)*T0(1,I)+S(11,29)*T0(2,I)+S(11,30)*T0(3,I) SHFL 9100
        DO 10 J=1,3 SHFL 9110
            S(11,J+24)=C1(I) SHFL 9120
    10 S(11,J+27)=C2(I) SHFL 9130
        DO 20 J=25,27 SHFL 9140
            J3=J+3 SHFL 9150
            DO 21 I=1,3 SHFL 9160
                T1=T0(1,I) SHFL 9170
                T2=T0(2,I) SHFL 9180
                T3=T0(3,I) SHFL 9190
                C1(I)=T1*S(25,J3)+T2*S(26,J3)+T3*S(27,J3) SHFL 9200
                C2(I)=T1*S(25,J3)+T2*S(26,J3)+T3*S(27,J3) SHFL 9210
    21 C3(I)=T1*S(28,J3)+T2*S(29,J3)+T3*S(30,J3) SHFL 9220
        DO 20 I=1,3 SHFL 9230
            S(I+24,J3)=C1(I) SHFL 9240
            S(I+24,J3)=C2(I) SHFL 9250
            S(I3,I+24)=C3(I) SHFL 9260
    20 S(I+27,J3)=C3(I) SHFL 9270
        DO 30 J=1,24 SHFL 9280
            DO 30 J=25,30 SHFL 9290
    30 S(J,I)=S(I,J) SHFL 9300
        RETURN SHFL 9310
        END SHFL 9320

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          SHARDHJNF SH2221 (SC,S2) SHFL 9330
C***** SHFL 9340
C-----COMPUTE K22 INVERSE * K21 FOR CONDENSATION OF GEOMETRIC STIFFNESS SHFL 9350
C      IN CASE OF QUADRILATERAL PLATE/SHELL ELEMENT SHFL 9360
C***** SHFL 9370
          IMPL(CIT REAR *R (A-H,O-Z) SHFL 9380
          DIMENSION SC(6,24),S2(30,30) SHFL 9390
          DO 710 I=25,30 SHFL 9400
              IF(S2(I,I).EQ.0.) GO TO 710 SHFL 9410
              PIV=1./S2(I,I) SHFL 9420
              DO 720 J=1,24 SHFL 9430
                  S2(I,J)=S2(I,J)*PIV SHFL 9440
              11=J+1 SHFL 9450
              IF(11.GT.30) GO TO 710 SHFL 9460
              DO 730 J=11,30 SHFL 9470
                  IF(S2(J,I).EQ.0.) GO TO 730 SHFL 9480
                  DO 740 K=1,24 SHFL 9490
                      S2(J,K)=S2(J,K)-S2(J,I)*S2(I,K) SHFL 9500
          720 CONTINUE SHFL 9510
          710 CONTINUE SHFL 9520
              DO 750 I=25,30 SHFL 9530
                  IF(S2(I,I).EQ.0.) GO TO 770 SHFL 9540
                  DO 760 J=1,24 SHFL 9550
                      SC(I-24,J)=S2(I,J) SHFL 9560
                  GO TO 750 SHFL 9570
          770 DO 780 J=1,24 SHFL 9580
              780 SC(I-24,J)=0. SHFL 9590
          750 CONTINUE SHFL 9600
          RETURN SHFL 9610
          END SHFL 9620

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      SHRRDITJNF DSHFL1(ANLD,ANFW,LNAD,NIIMDV) ) SHFL 9630
C***** SHFL 9640
C-----DEFIGN OF PLATE/SHFL1 ELEMENTS FOR STRESS CONSTRAINTS SHFL 9650
C***** SHFL 9660
      NIMFNSJON ANLD(NIIMDV),ANFW(NIIMDV),LNAD(NIIMDV) SHFL 9670
      CMMON/JUNK/ LT,LH,L,SG(20),FX,FY,FXY,SMX,SMY,SMXY,SIG, SHFL 9680
1 IDVAR,TFX,ERC,H,XINFR1,TFN,COMP,SHFAR,BETA,HP(2),JUN1(327) SHFL 9690
      FXY=FXY/(SHFAR*H) SHFL 9700
      SMXY=6.0*SMXY/(SHFAR*H*H) SHFL 9710
      CC=-1.0 SHFL 9720
      DO 200 I=1,2 SHFL 9730
      IF(I,FO,2) CC=1.0 SHFL 9740
      C1=FX/H+CC*6.0*SMX/(H*H) SHFL 9750
      C2=FY/H+CC*6.0*SMY/(H*H) SHFL 9760
      AX=TFN SHFL 9770
      AY=TFN SHFL 9780
      IF(C1,LT,0.) AX=COMP SHFL 9790
      IF(C2,LT,0.) AY=COMP SHFL 9800
      FX1=FX/(AX*H) SHFL 9810
      FY1=FY/(AY*H) SHFL 9820
      SMX1=6.0*SMX/(AX*H*H) SHFL 9830
      SMY1=6.0*SMY/(AY*H*H) SHFL 9840
      CXX= FX1*FX1+FY1*FY1+FXY1*FXY1-FX1*FY1 SHFL 9850
      CX=2.0*(FX1*SMX1+FY1*SMY1+FXY1*SMXY1)-FX1*SMY1-FY1*SMX1 SHFL 9860
      CX=CX*CC SHFL 9870
      C= SMX1*SMX1+SMY1*SMY1+SMXY1*SMXY1-SMX1*SMY1 SHFL 9880
      HP(I)=H SHFL 9890
      DO 100 J=1,10 SHFL 9900
      HHH=(CXX+CX*H/HP(I))*0.5 SHFL 9910
      HHH=HHH+SQRT(HHH*HHH+C) SHFL 9920
      HHH=SQRT(HHH)*H SHFL 9930
      IF(ABS(HHH-HP(I)),LT,0.001) GO TO 200 SHFL 9940
100 HP(I)=HHH SHFL 9950
200 HP(I)=HHH SHFL 9960
      HH=HP(I) SHFL 9970
      IF(HH,LT,HP(2)) HH=HP(2) SHFL 9980
      HH=HH/ERC SHFL 9990
      IF(HH,LE,ANFW(IDVAR)) GO TO 400 SHFL 0000
      ANFW(IDVAR) =HH SHFL 0010
      LNAD(IDVAR) =L SHFL 0020
400 RETURN SHFL 0030
      FMD SHFL 0040

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      SUBROUTINE SHFLG1(SC, ,TD,NTRI)
      SHFL0050
C *****
C-----CALCULATE UNIT GEOMETRIC STIFFNESS MATRICES OF SHFL ELEMENT
      SHFL0060
C *****
      IMPLICIT REAL*8 (A-H,O-Z)
      SHFL0090
      DIMENSION SC(6,24),TD(9)
      SHFL0100
      COMMON/COMPL/A(12),R(12),T1(36),T2(36),T3(36),LOC(12),G1(9,9),
      SHFL0110
      1 G2(9,9),G3(9,9),C1(3),C2(3),C3(3),COM(36)
      SHFL0120
      COMMON/FM/LM(24),GU1(30,30),GU2(30,30),GU3(30,30),EM1(62)
      SHFL0130
      DO 200 I=1,30
      SHFL0140
      DO 200 J=1,30
      SHFL0150
      GU1(I,J)=0.
      SHFL0160
      GU2(I,J)=0.
      SHFL0170
      DO 200 GU3(I,J)=0.
      SHFL0180
      DO 100 NT=1,NTRI
      SHFL0190
C *****
C-----FORM UNIT GEOMETRIC STIFFNESS MATRICES IN LOCAL COORDINATES
      SHFL0200
C *****
      CALL SHFLG2(NT)
      SHFL0230
C *****
C-----TRANSFORM TO GLOBAL COORDINATES
      SHFL0240
C *****
      CALL SHLC11(GU1,G1,T1,T2,T3,LOC,NT)
      SHFL0270
      CALL SHLC11(GU2,G2,T1,T2,T3,LOC,NT)
      SHFL0280
      100 CALL SHLC11(GU3,G3,T1,T2,T3,LOC,NT)
      SHFL0290
      IF (NTRI.EQ.1) RETURN
      SHFL0300
C *****
C-----CHECK FOR FLAT OR NEARLY FLAT QUADRILATERAL ELEMENT AND
      SHFL0310
C TRANSFORM STIFFNESS AT 5TH NODE TO GLOBAL COORDINATES
      SHFL0320
C *****
      CALL SHLC12(GU1,TD,C1,C2,C3)
      SHFL0350
      CALL SHLC12(GU2,TD,C1,C2,C3)
      SHFL0360
      CALL SHLC12(GU3,TD,C1,C2,C3)
      SHFL0370
C *****
C-----CONDENSATION OF INTERNAL DEGREES OF FREEDOM OF UNIT GEOMETRIC
      SHFL0380
C STIFFNESS MATRICES
      SHFL0390
C *****
      CALL SHLC02(GU1,SC)
      SHFL0420
      CALL SHLC02(GU2,SC)
      SHFL0430
      CALL SHLC02(GU3,SC)
      SHFL0440
      RETURN
      SHFL0450
      END
      SHFL0460

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SHRDLIN SHFLG2(NT)                                SHFI 0470
C*****SHFI 0480
C-----FIND UNIT GEOMETRIC STIFFNESS MATRICES CORRESPONDING TO SHFI 0490
C      MEMBRANE STRESS RESULTANTS N(X),N(Y) AND N(XY) IN LOCAL SHFI 0500
C      COORDINATES FOR THE TRIANGULAR ELEMENT COMPONENT SHFI 0510
C*****SHFI 0520
      IMPLCJT REFA,*R (A-H,N-2) SHFI 0530
      DIMENS ION 7(7,3),H(7),IPM(3) SHFI 0540
      COMMON/COMPL/A(3,4),R(3,4),IT(108),LOC(12),G1(9,9),G2(9,9),G3(9,9) SHFI 0550
      I,N(3,9),NX(9),NY(9) SHFI 0560
      DATA 7/.3333333333333333 ,0.059715871789770 ,0.470142064105115 , SHFI 0570
      1 0.470142064105115 ,.797426985353087 ,0.101286507323456 , SHFI 0580
      2 0.101286507323456 ,0.333333333333333 ,0.470142064105115 , SHFI 0590
      3 0.059715871789770 ,0.470142064105115 ,.101286507323456 , SHFI 0600
      4 0.797426985353087 ,0.101286507323456 ,0.333333333333333 , SHFI 0610
      5 0.470142064105115 ,0.470142064105115 ,.059715871789770 , SHFI 0620
      6 0.101286507323456 ,0.101286507323456 ,0.797426985353087 / SHFI 0630
      DATA H/.225,.132394152788506 ,.132394152788506 ,.132394152788506 ,SHFI 0640
      1 .125939180544827 ,.125939180544827 ,.125939180544827 /SHFI 0650
      DATA IPM/2,3,1/ SHFI 0660
      DO 5 I=1,9 SHFI 0670
      DO 5 J=1,9 SHFI 0680
      G1(I,J)=0. SHFI 0690
      G2(I,J)=0. SHFI 0700
      5 G3(J,J)=0. SHFI 0710
      A1=A(1,NT) SHFI 0720
      A2=A(2,NT) SHFI 0730
      A3=A(3,NT) SHFI 0740
      R1=R(1,NT) SHFI 0750
      R2=R(2,NT) SHFI 0760
      R3=R(3,NT) SHFI 0770
      REFA4=(A3*R2-A2*R3)*2 SHFI 0780
      DO 100 IP=1,7 SHFI 0790
      H1=H(IP) SHFI 0800
      C*****SHFI 0810
      C-----FIND SHAPE FUNCTION DERIVATIVES W.R.T NATURAL COORDINATES (D) AND SHFI 0820
      C      W.R.T X AND Y COORDINATES (DX,DY) AT INTEGRATION POINT IP SHFI 0830
      C*****SHFI 0840
      DO 10 I=1,3 SHFI 0850
      J=IPM(I) SHFI 0860
      K=IPM(J) SHFI 0870
      I1=I*3-2 SHFI 0880
      J1=J*3-2 SHFI 0890
      KK=K*3-2 SHFI 0900
      A1=A(I,NT) SHFI 0910
      A11=A(J,NT) SHFI 0920
      AK=A(K,NT) SHFI 0930
      R1=R(I,NT) SHFI 0940
      R11=R(J,NT) SHFI 0950
      RK=R(K,NT) SHFI 0960
      Z1=Z(IP,I) SHFI 0970
      Z2=Z(IP,J) SHFI 0980
      Z3=Z(IP,K) SHFI 0990
      Z11=Z1*71 SHFI 1000
      Z22=Z2*72 SHFI 1010
      Z33=Z3*73 SHFI 1020
      Z12=Z1*72 SHFI 1030
      Z23=Z2*73 SHFI 1040
      Z13=Z1*73 SHFI 1050
      D(I,I1 J)=1,0-Z22-Z33+2*(Z12+Z13) SHFI 1060

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      D(I,I+1)=2*(R,I*Z13-RK*Z12)+0.5*(R,I-RK)*Z23      SHEF 1070
      D(I,I+2)=2*(A,I*Z13-AK*Z12)+0.5*(A,I-AK)*Z23      SHEF 1080
      D(I,J,I)=722-2*Z12      SHEF 1090
      D(I,J,I+1)=RK*722+0.5*(RK-R1)*Z23      SHEF 1100
      D(I,J,I+2)=AK*722+0.5*(AK-A1)*Z23      SHEF 1110
      D(I,KK)=733-2*Z13      SHEF 1120
      D(I,KK+1)=-R,I*733+0.5*(R,I-R,I)*Z23      SHEF 1130
      10 D(I,KK+2)=-A,I*733+0.5*(A,I-A,I)*Z23      SHEF 1140
      DO 20 J=1,9      SHEF 1150
      DX(I)=D(I,I)*R1+D(I,2)*R2+D(I,3)*R3      SHEF 1160
      20 DY(I)=D(I,I)*A1+D(I,2)*A2+D(I,3)*A3      SHEF 1170
C*****SHEF 1180
C-----FIND CONTEGRATION POINTS IN GEOMETRIC STIFFNESS MATRIX FROM      SHEF 1190
C      INTEGRATION POINTS      SHEF 1200
C*****SHEF 1210
      DO 30 I=1,9      SHEF 1220
      DO 30 J=1,I      SHEF 1230
      G1(I,J)=G1(I,J)+DX(I)*DY(J)*H1      SHEF 1240
      G2(I,J)=G2(I,J)+DY(I)*DY(J)*H1      SHEF 1250
      30 G3(I,J)=G3(I,J)+(DX(I)*DY(J)+DY(I)*DX(J))*H1      SHEF 1260
      100 CONTINUE      SHEF 1270
      DO 40 I=1,9      SHEF 1280
      DO 40 J=1,I      SHEF 1290
      G1(I,J)=G1(I,J)/ARFA4      SHEF 1300
      G2(I,J)=G2(I,J)/ARFA4      SHEF 1310
      G3(I,J)=G3(I,J)/ARFA4      SHEF 1320
      G1(J,I)=G1(I,J)      SHEF 1330
      G2(J,I)=G2(I,J)      SHEF 1340
      40 G3(J,I)=G3(I,J)      SHEF 1350
      RETURN      SHEF 1360
      END      SHEF 1370

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      SUBROUTINE SHLCND2(G,SC)      SHEF 1380
C*****SHEF 1390
C-----CONDENSATION OF INTERNAL D.O.F OF GEOMETRIC STIFFNESS MATRIX      SHEF 1400
C*****SHEF 1410
      IMPLICIT REAL*8 (A-H,O-Z)      SHEF 1420
      DIMENSION D(30,30),SC(6,24)      SHEF 1430
      DO 50 I=25,30      SHEF 1440
      DO 50 J=1,24      SHEF 1450
      SUM=0.      SHEF 1460
      DO 60 K=25,30      SHEF 1470
      60 SUM=SUM+G(I,K)*SC(K-24,J)      SHEF 1480
      50 G(I,J)=-SUM+G(I,J)      SHEF 1490
      DO 70 I=1,24      SHEF 1500
      DO 70 J=1,I      SHEF 1510
      SUM=0.      SHEF 1520
      DO 80 K=1,6      SHEF 1530
      80 SUM=SUM-SC(K,I)*G(K+24,J)-G(I,K+24)*SC(K,J)      SHEF 1540
      G(I,J)=G(I,J)+SUM      SHEF 1550
      70 G(J,I)=G(I,J)      SHEF 1560
      RETURN      SHEF 1570
      END      SHEF 1580

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C ***** SUBROUTINE CLAMP ( I0,X,Y,Z,NUMP,NUNMP) ***** ROUN0250
C ***** BOUNDARY ELEMENT MATRIXCES ***** ROUN0260
C ***** ***** ROUN0270
C ***** ***** ROUN0280
C ***** ***** ROUN0290
C ***** ***** ROUN0300
C ***** ***** ROUN0310
C ***** ***** ROUN0320
C ***** ***** ROUN0330
C ***** ***** ROUN0340
C ***** ***** ROUN0350
C ***** ***** ROUN0360
C ***** ***** ROUN0370
C ***** ***** ROUN0380
C ***** ***** ROUN0390
C ***** ***** ROUN0400
C ***** ***** ROUN0410
C ***** ***** ROUN0420
C ***** ***** ROUN0430
C ***** ***** ROUN0440
C ***** ***** ROUN0450
C ***** ***** ROUN0460
C ***** ***** ROUN0470
C ***** ***** ROUN0480
C ***** ***** ROUN0490
C ***** ***** ROUN0500
C ***** ***** ROUN0510
C ***** ***** ROUN0520
C ***** ***** ROUN0530
C ***** ***** ROUN0540
C ***** ***** ROUN0550

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C*****H0UN0550
      WRITF(IW,2001)
      N=1
200 READ(IR,1000) IFL,IF,KD,KR,INC,SD,SR,TRACEF
      IF(KD.NF.1) KD=0
      IF(KR.NF.1) KR=0
      IF(INC.F0.0) INC=1
      IF(TRACEF.F0.0) TRACEF= 1.0F 10
      KK=INC*(IFL-N)
      DO 100 J=1,5
100  IX(I)=IF(I)-KK
      IF(JF(2).NF.0) GO TO 210
      DO 101 I=3,5
101  IX(I)=0
210  DO 550 NFI=N,IFL
      DO 110 I=1,5
      II=IX(I)
      IF(II.F0.0) GO TO 110
      XX(I)=X(II)
      YY(I)=Y(II)
      ZZ(I)=Z(II)
110  CONTINUE
      IF(IX(3).F0.0) GO TO 250
      CALL VECTDR(H,XX(2),YY(2),Z(2),XX(3),YY(3),Z(3))
      CALL VECTDR(V,XX(4),YY(4),Z(4),XX(5),YY(5),Z(5))
      CALL CROSS(U,V,T)
      DO TO 260
250  CALL VECTDR(T,XX(1),YY(1),Z(1),XX(2),YY(2),Z(2))
260  DO 50 J=1,3
      ST(1,J)=T(1,J)*TRACEF*KD
      ST(2,J+3)=T(1,J)*TRACEF*KR
      R(1,J)=T(1,J)*TRACEF*SD*KD
      R(1+3,J)=T(1,J)*TRACEF*SR*KR
      DO 50 J=1,3
      S(1,J)=T(1,J)*T(1,J)*TRACEF*KD
50  S(1+3,J+3)=T(1,J)*T(1,J)*TRACEF*KR
      DO 500 I=2,6
      II=I-1
      DO 500 J=1,11
500  S(II,J)=S(J,I)
      DO 520 J=1,4
      TT(1,J)=-TRACEF*KD*SD*FMUL(J)
      TT(2,J)=-TRACEF*KR*SR*FMUL(J)
      DO 520 I=1,6
520  P(1,J)=R(1)*FMUL(J)
      II=IX(1)
      DO 600 J=1,6
600  LM(I)=JDI(II,I)
      CALL CALRAN(MDIF,I,M,S,P,ST,II,NH,MV,NS,ND,NW,INDV,IFX,FRC)
      WRITF(IW,200) NFI,IX,KD,KR,SD,SR,TRACEF
      IX(1)=IX(1)+INC
      IX(2)=IX(2)+INC
      IF(IX(3).F0.0) GO TO 550
      DO 650 I=3,5
650  IX(I)=IX(I)+INC
550  CONTINUE
      N=IFL+1
      IF(N.IF.NHMF) GO TO 200
      RETURN
1000 FORMAT(9I5,5X,3F10.0)

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```

R0UN0560
R0UN0570
R0UN0580
R0UN0590
R0UN0600
R0UN0610
R0UN0620
R0UN0630
R0UN0640
R0UN0650
R0UN0660
R0UN0670
R0UN0680
R0UN0690
R0UN0700
R0UN0710
R0UN0720
R0UN0730
R0UN0740
R0UN0750
R0UN0760
R0UN0770
R0UN0780
R0UN0790
R0UN0800
R0UN0810
R0UN0820
R0UN0830
R0UN0840
R0UN0850
R0UN0860
R0UN0870
R0UN0880
R0UN0890
R0UN0900
R0UN0910
R0UN0920
R0UN0930
R0UN0940
R0UN0950
R0UN0960
R0UN0970
R0UN0980
R0UN0990
R0UN1000
R0UN1010
R0UN1020
R0UN1030
R0UN1040
R0UN1050
R0UN1060
R0UN1070
R0UN1080
R0UN1090
R0UN1100
R0UN1110
R0UN1120
R0UN1130
R0UN1140

```

1005 FORMAT (4F10.0)	BOUND1150
2000 FORMAT(24H1 B O U N D A R Y E L E M E N T S //	BOUND1160
1 23H NUMBER OF ELEMENTS =,15 )	BOUND1170
2001 FORMAT(//22H BOUNDARY ELEMENT DATA //	BOUND1180
1 5X,5HCONST,5X,4HNODE,42H /--NODES DEFINING CONSTRAINT DIRECTION--	BOUND1190
2/,5X,5HNODES, 8X,5HDISPL,5X,8HROTATION,4X,5HSTIFF /	BOUND1200
3 4X,6HNUMBER,6X,1HN,8X,2HNI,8X,2HNI,8X,2HNI,8X,2HNI,8X,2HNI,6X,2HND,3X,	BOUND1210
4 2HNR,11X,1HD,11X,1HR,11X,1HS )	BOUND1220
2005 FORMAT (// 25H ELEMENT LOAD MULTIPLIERS//	BOUND1230
. 9X,1HA,9X,1HR,9X,1HC,9X,1HD /4F10.4)	BOUND1240
2100 FORMAT(17,5110,3X,215,5X,1P3F12.2)	BOUND1250
END	BOUND1260

```

SUBROUTINE TRUSS (A,MTOT)
  DIMENSION A(MTOT)
  WRITE(6,202)
  STOP
202 FORMAT(1X,' PROGRAM ENTERED DUMMY SUBROUTINE TRUSS      '///)
END

```

```

SUBROUTINE BEAM (A,MTOT)
  DIMENSION A(MTOT)
  WRITE (6,202)
  STOP
202 FORMAT(1X,' PROGRAM ENTERED DUMMY SUBROUTINE BEAM      '///)
END

```

```

SUBROUTINE PLANE (A,MTOT)
  DIMENSION A(MTOT)
  WRITE (6,202 )
  STOP
202 FORMAT(1X,' PROGRAM ENTERED DUMMY SUBROUTINE PLANE      '///)
END

```

```

SUBROUTINE SHEAR (A,MTOT)
  DIMENSION A(MTOT)
  WRITE (6,202 )
  STOP
202 FORMAT(1X,' PROGRAM ENTERED DUMMY SUBROUTINE SHEAR      '///)
END

```

```

SUBROUTINE SHELL(A,MTOT)
  DIMENSION A(MTOT)
  WRITE(6,202)
  STOP
202 FORMAT(1X,' PROGRAM ENTERED DUMMY SUBROUTINE SHELL      '///)
END

```

```

SUBROUTINE ROUND(A,MTOT)
  DIMENSION A(MTOT)
  WRITE(6,202)
  STOP
202 FORMAT(1X,' PROGRAM ENTERED DUMMY SUBROUTINE ROUND      '///)
END

```